# A symbolic framework for flexible multibody systems applied to horizontal-axis wind turbines 

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#### Abstract

The article presents a symbolic framework that is used to obtain the linear and nonlinear equations of motion of a multibody system including rigid and flexible bodies. Our approach is based on Kane's method and a nonlinear shape function representation for flexible bodies. The method yields compact symbolic equations of motion with implicit account of the constraints. The general and automatic framework facilitate facilitates the creation and manipulation of models with various levels of fidelity. The symbolic treatment allows for the obtention of analytical gradients and linearized equations of motion. The linear and nonlinear equations can be exported to Python code or dedicated software. The applieation are multiple such as: time-domain-There are multiple applications, such as time domain simulation, stability analyses, frequency domain analyses, advanced controller design, state observers, digital twins, eteand digital twins. In this article, we describe the method we used to systematically generate the equations of motion of multibody systems. We apply the framework to generate illustrative landbased and offshore wind turbine models. We compare our results with OpenFAST simulations and discuss the advantages and limitations of the method. A The Python implementation is provided as an open-source project.


## 1 Introduction

The next generation of wind turbine digital technologies requires versatile aero-servo-hydro-elastic models, with various levels of fidelity, suitable for a wide range of applications. Such applications include $\div$ time domain simulations, linearization (for controller design and tuning, or frequency domain analyses), analytical gradients (for optimization procedures), and generation of dedicated, high-performance or embedded code (for stand-alone simulations, state observers or digital twins). Current models are implemented for a specific purpose and are usually based on an heuristic structure. Aeroelastic tools, such as Flex (Øye, 1983; Branlard, 2019) or ElastoDyn $(?)($ Jonkman et al., 2021), rely on $\div$ an assumed chain of connections between bodies, a given set of degrees of freedom, and predefined orientations of shape functions.

Tools with linearization capabilities, such as hawestab2HAWCStab2 (Sønderby and Hansen, 2014) or OpenFAST (?) (Jonkman et al., 2021) are dedicated to horizontal-axis wind turbines, and the evaluation of the gradients are limited to hardcoded analytical expressions or numerical finite differences. Small implementation changes often require extensive redevelopment, and the range of applications of the tools remain remains limited (Simani, 2015).

To address this issue, we propose a framework for the automatic derivation, processingand parametrization, and parameterization of models with granularity in the level of fidelity. Our approach is based on Kane's method (Kane and Wang, 1965) and a nonlinear shape function representation of flexible bodies (Shabana, 2013) described using a standard input data (SID) format (Wallrapp, 1994; Schwertassek and Wallrapp, 1999). The method yields compact symbolic equations of motion with implicit account of the constraints. Similar approaches have been presented in the literature: Kurz and Eberhard (2009), Merz (2018), Lemmer (2018), and Branlard (2019). Our framework differs in the fact that all equations are processed at a symbolic level and therefore the model can be used in its nonlinear or linearized form. We implemented an open-source version in Python using SymPy (SymPy, 2021), leveraging its mechanical toolbox. Alternative symbolic frameworks found in the literature are usually limited to rigid bodies (Verlinden et al., 2005; Kurz and Eberhard, 2009; Gede et al., 2013; Docquier et al., 2013) ,-or are closed-source (Reckdahl and Mitiguy, 1996; Kurtz et al., 2010; MotionGenesis, 2016) , and cannot be directly processed in Python.

Kane's method and the nonlinear shape function approach presented in this article do not represent the state of the art of multibody dynamics with flexible bodies. The geometrically exact beam theory (Simo, 1985; Jelenić and Crisfield, 1999; Géradin and Card is more precise than the shape function approach. Similarly, multipurpose multibody software exists (Lange et al., 2007), such as ANSYS (ANSYS, 2022), SIMPACK (SIMPACK, 2022) or MBDyn (MBDyn, 2022). These more advanced approaches target different applications than those envisioned in this study: they are suitable for numerical simulations, but they cannot provide simple and computationally efficient nonlinear and linear models.

In section 2, we present the formalism used to derive the equations of motion. In section 3, we given an overview of how the equations were implemented into a symbolic calculation framework, to easily manipulate the equations and generate dedicated code. Example of applications relevant to wind energy are given in section 4. Discussions and conclusions follow.

## 2 Method to obtain the equations of motion

In this section, we present the formalism used to setup the equations of motion.

### 2.1 System definition and kinematics

We consider a system of $n_{b}$ bodies, rigid or flexible, connected by a set of joints. For simplicity, we assume that no kinematic loops are present in the system, and the masses of the bodies are constant. An inertial frame is defined to express the positions, velocities, and accelerations of the bodies. We adopt a minimal set of generalized coordinates, $\boldsymbol{q}$, of dimension $n_{q}$, to describe the kinematics of the bodies: joint coordinates describing the joints displacements, and Rayleigh-Ritz coordinates for the amplitudes of the shape functions of the flexible bodies (see, e.g., Branlard (2019)). The choice of coordinates is left to the user, but it is assumed to form a minimal set. We will provide illustrative examples in section 4.

At a given time, the positions, orientations, velocities, and accelerations of all the points of the structure are entirely determined by the knowledge of $\boldsymbol{q}, \dot{\boldsymbol{q}} \boldsymbol{q}_{2} \dot{\boldsymbol{q}}_{2}$ and $\ddot{\boldsymbol{q}}$, where $\left({ }^{\circ}\right)$ represents the time derivative. For a given body $i$, and a point $P$
belonging to the body, the position, velocity, ${ }_{2}$ and acceleration of the point are given by (see, e.g., Shabana (2013)):
$\boldsymbol{r}_{P}=\boldsymbol{r}_{i}+\boldsymbol{s}_{P}=\boldsymbol{r}_{i}+\boldsymbol{s}_{P_{0}}+\boldsymbol{u}_{P}$
$\boldsymbol{v}_{P}=\boldsymbol{v}_{i}+\boldsymbol{\omega}_{i} \times \boldsymbol{s}_{P}+\left(\dot{\boldsymbol{u}}_{P}\right)_{i}$
$\boldsymbol{a}_{P}=\boldsymbol{a}_{i}+\boldsymbol{\omega}_{i} \times\left(\boldsymbol{\omega}_{i} \times \boldsymbol{s}_{P}\right)+\dot{\boldsymbol{\omega}}_{i} \times \boldsymbol{s}_{P}+2 \boldsymbol{\omega}_{i} \times\left(\dot{\boldsymbol{u}}_{P}\right)_{i}+\left(\ddot{\boldsymbol{u}}_{P}\right)_{i}$
where $: \boldsymbol{r}_{i}, \boldsymbol{v}_{i}$, and $\boldsymbol{a}_{i}$ are the position, velocity, and acceleration of the origin of the body, respectively; $\boldsymbol{s}_{P_{0}}$ is the initial (undeformed) position vector of point $P$ with respect to the body origin; the subscript $P$ is used for the deformed position of the point and $P_{0}$ for the underformed position; $\boldsymbol{u}_{P}$ is the elastic displacement of the point (equal to 0 for rigid bodies); $\boldsymbol{\omega}_{i}$ is the rotational velocity of the body with respect to the inertial frame; $\left(^{\circ}\right)$ and $\left({ }^{\circ}\right)_{i}$ refer to time derivatives in the inertial and body frame respectively. Throughout the article, we use bold symbols for vectors and matrices, and uppercase symbols for most matrices. The elastic displacement is obtained as a superposition of elastic deformations (see subsection 2.4). We define the transformation matrix $\boldsymbol{R}_{i}$ that transforms coordinates from the body frame to the inertial frame, and by definition $\left[\tilde{\boldsymbol{\omega}}_{i}\right]=\dot{\boldsymbol{R}}_{i} \boldsymbol{R}_{i}^{T}$, where [ $\left.{ }^{\sim}\right]$ represents the skew symmetric matrix-, and the exponent $T$ denotes the matrix transpose. We assume that vectors are represented as column vectors to conveniently introduce matrix-vector multiplications. We use the notation "." to indicate the dot product between two vectors (irrespective of their column or row representation).

### 2.2 Introduction to Kane's method

Kane's method (Kane and Wang, 1965) is a powerful and systematic way to obtain the equations of motion of a system. The procedure leads to $n_{q}$ coupled equations of motion:
$\mathrm{f}_{r}+\mathrm{f}_{r}^{*}=0, \quad r=1 \ldots n_{q}$
where $\mathrm{f}_{r}^{*}$ is associated with inertial loads and $\mathrm{f}_{r}$ is associated with external loads, and these components are obtained for each all generalized coordinates. The components are obtained as a superposition of contributions from each body:
$\mathrm{f}_{r}=\sum_{i=1}^{n_{b}} \mathrm{f}_{r i}, \quad \mathrm{f}_{r}^{*}=\sum_{i=1}^{n_{b}} \mathrm{f}_{r i}^{*}$
The terms $\mathrm{f}_{r i}$ and $\mathrm{f}_{r i}^{*}$ can be obtained for each body individually and assembled at the end to form the final system of equations. We will present in subsection 2.3 and subsection 2.4 how these terms are defined for rigid bodies and flexible bodies, ${ }_{2}$ respectively.

### 2.3 Rigid bodies

We assume that body $i$ is a rigid body and proceed to define the terms $\mathrm{f}_{r i}$ and $\mathrm{f}_{r i}^{*}$. The inertial force, $\boldsymbol{f}_{i}^{*}$, and inertial torque, $\tau_{i}^{*}$, acting on the body are:
$\boldsymbol{f}_{i}^{*}=-m_{i} \boldsymbol{a}_{G, i}, \quad \boldsymbol{\tau}_{i}^{*}=-\boldsymbol{I}_{G, i} \cdot \dot{\boldsymbol{\omega}}_{i}-\boldsymbol{\omega}_{i} \times\left(\boldsymbol{I}_{G, i} \cdot \boldsymbol{\omega}_{i}\right)$
where $m_{i}$ is the mass of the body, $\boldsymbol{a}_{G, i}$ is the acceleration of its center of mass with respect to the inertial frame, and $\boldsymbol{I}_{G, i}$ is the inertial tensor of the body expressed at its center of mass. Equation 6 is a vectorial relationship;; it may therefore be evaluated in any coordinate system. The component $f_{r i}^{*}$ is defined as:
$\mathrm{f}_{r i}^{*}=\boldsymbol{J}_{v, r i} \cdot \boldsymbol{f}_{i}^{*}+\boldsymbol{J}_{\omega, r i} \cdot \boldsymbol{\tau}_{i}^{*}$
with
$\boldsymbol{J}_{v, r i}=\frac{\partial \boldsymbol{v}_{G, i}}{\partial \dot{q}_{r}}, \quad \boldsymbol{J}_{\omega, r i}=\frac{\partial \boldsymbol{\omega}_{i}}{\partial \dot{q}_{r}}$
where $\boldsymbol{v}_{G, i}$ is the velocity of the body masseenter-mass center with respect to the inertial frame. The partial velocities, or Jacobians, $\boldsymbol{J}_{v}$ and $\boldsymbol{J}_{\omega}$, are key variables of the Kane's method. They project the physical coordinates into the generalized coordinates $(\boldsymbol{q})$, inherently accounting for the kinematic constraints between bodies. In numerical implementations, the Jacobians are typically stored in matricial forms, referred to as "velocity transformation matrices." The terms $\mathrm{f}_{r i}^{*}$ can equivalently be obtained using the partial velocity of any body point (e.g., the origin) by carefully transferring the inertial loads to the chosen point. The external forces and torques acting on the body are combined into an equivalent force and torque acting at the center of mass, written as $\boldsymbol{f}_{i}$ and $\boldsymbol{\tau}_{i}$. The component $\mathrm{f}_{r i}$ is then given by:
$\mathrm{f}_{r i}=\boldsymbol{J}_{v, r i} \cdot \boldsymbol{f}_{i}+\boldsymbol{J}_{\omega, r i} \cdot \boldsymbol{\tau}_{i}$

Equivalently, the contributions from each individual force, $\boldsymbol{f}_{i, j}$, acting on a point $P_{j}$ of the body $i$, and each individual torquestorque, $\boldsymbol{\tau}_{i, k}$, can be summed using the appropriate partial velocity to obtain $\mathrm{f}_{r i}$ :
$\mathrm{f}_{r i}=\sum_{j} \frac{\partial \boldsymbol{v}_{P_{j}}}{\partial \dot{q}_{r}} \cdot \boldsymbol{f}_{i, j}+\sum_{k} \boldsymbol{J}_{\omega, r i} \cdot \boldsymbol{\tau}_{i, k}$
where $\boldsymbol{v}_{P_{j}}$ is the velocity of the point $j$ with respect to the inertial frame. Equation 7 and Equation 9 are inserted in into Equation 5 to obtain the final equations of motion.

### 2.4 Flexible bodies

We assume that body $i$ is a flexible body and proceed to define the terms $\mathrm{f}_{r i}$ and $\mathrm{f}_{r i}^{*}$. The dynamics of a flexible body is are described in standards textbooks such as Shabana (2013) or Schwertassek and Wallrapp (1999). Unlike rigid bodies, the equations for flexible bodies are typically expressed with respect to a reference point different from the center of mass. We will call this point the origin and write it $O_{i}$. The elastic displacement field of the body is written as $\boldsymbol{u}$. It defines the displacement of any point of the body with respect to its undeformed position. Using the first-orderzeroth-order ${ }^{1}$ Rayleigh-Ritz approximation, the displacement field at a given point, $P$, is given by the sum of shape function contributions: $\boldsymbol{u}(P)=\sum_{j=1}^{n_{e, i}} \boldsymbol{\Phi}_{i j}(P) \boldsymbol{q}_{e, i j}(t)$, where $\boldsymbol{\Phi}_{i j}$ are the shape functions (displacement fields) of body $i_{2}$ and $\boldsymbol{q}_{e, i j}$ is the subset of $\boldsymbol{q}$ consisting of the elastic coordinates of body $i$, of size $n_{e, i}$. The principles of the shape function approach applied to beams are given in Appendix B. The

[^0]shape functions are more easily represented in the body coordinate system. Vectors and matrices that are explicitly written in the body frame will be written with primes. The equations of motion of the flexible bodies are (Wallrapp, 1994):
\[

\left[$$
\begin{array}{ccc}
\boldsymbol{M}_{x x}^{\prime} & \boldsymbol{M}_{x \theta}^{\prime} & \boldsymbol{M}_{x e}^{\prime}  \tag{11}\\
& \boldsymbol{M}_{\theta \theta}^{\prime} & \boldsymbol{M}_{\theta e}^{\prime} \\
\text { sym. } & & \boldsymbol{M}_{e e}^{\prime}
\end{array}
$$\right]_{i}\left[$$
\begin{array}{c}
\boldsymbol{a}_{i}^{\prime} \\
\dot{\boldsymbol{\omega}}_{i}^{\prime} \\
\ddot{\boldsymbol{q}}_{e, i}
\end{array}
$$\right]_{\omega, e}+\left[$$
\begin{array}{c}
\boldsymbol{k}_{\omega, x}^{\prime} \\
\boldsymbol{k}_{\omega, \theta}^{\prime} \\
\boldsymbol{k}_{\omega}^{\prime}
\end{array}
$$\right]_{i}+\left[$$
\begin{array}{c}
0 \\
0 \\
\boldsymbol{k}_{e}
\end{array}
$$\right]_{i}=\left[$$
\begin{array}{c}
\boldsymbol{f}_{x}^{\prime} \\
\boldsymbol{f}_{\theta}^{\prime} \\
\boldsymbol{f}_{e}
\end{array}
$$\right]_{i}
\]

with:-where the $x, \theta$, and $e$, subscripts that-respectively indicate the translation, rotation, and elastic components; $\boldsymbol{M}$, is the mass matrix of dimension $6+n_{e, i}$ made of the block matrices $\boldsymbol{M}_{x x}, \cdots, \boldsymbol{M}_{e e} ; \boldsymbol{a}_{i}$ and $\dot{\boldsymbol{\omega}}_{i}$,-are the linear and angular acceleration of the body (origin) with respect to the inertial frame; $\boldsymbol{k}_{\omega}$, are the centrifugal, gyration, and Coriolis loads, also called quadratic velocities-velocity loads; $\boldsymbol{k}_{e}$, are the elastic strain loads, which may contain geometric stiffening effects; $\boldsymbol{f}$ , are the external forces, torques, and elastic generalized forces. The different components of $\boldsymbol{M}, \boldsymbol{k}_{\omega}, \boldsymbol{k}_{e}$, and $\boldsymbol{f}$ are given in Appendix A. These terms depend on $\boldsymbol{q}, \dot{\boldsymbol{q}}$, and $\boldsymbol{\Phi}_{i}$. The inertial force, torque, and elastic loads are:
$\boldsymbol{f}_{i}^{*}=-\boldsymbol{R}_{i}\left[\boldsymbol{M}_{x x}^{\prime} \boldsymbol{a}_{i}^{\prime}+\boldsymbol{M}_{x \theta}^{\prime} \dot{\boldsymbol{\omega}}_{i}^{\prime}+\boldsymbol{M}_{x e} \ddot{\boldsymbol{q}}_{e, i}+\boldsymbol{k}_{\omega, x}^{\prime}\right]$
$\boldsymbol{\tau}_{i}^{*}=-\boldsymbol{R}_{i}\left[\boldsymbol{M}_{\theta x}^{\prime} \boldsymbol{a}_{i}^{\prime}+\boldsymbol{M}_{\theta \theta}^{\prime} \dot{\boldsymbol{\omega}}_{i}^{\prime}+\boldsymbol{M}_{\theta e} \ddot{\boldsymbol{q}}_{e, i}+\boldsymbol{k}_{\omega, \theta}^{\prime}\right]$
$\boldsymbol{h}_{i}^{*}=-\quad\left[\boldsymbol{M}_{e x}^{\prime} \boldsymbol{a}_{i}^{\prime}+\boldsymbol{M}_{e \theta}^{\prime} \dot{\boldsymbol{\omega}}_{i}^{\prime}+\boldsymbol{M}_{e e} \ddot{\boldsymbol{q}}_{e, i}+\boldsymbol{k}_{\omega, e}^{\prime}\right]$
The external and elastic loads are:
$\boldsymbol{f}_{i}=\boldsymbol{R}_{i} \boldsymbol{f}_{x}^{\prime}$
$\boldsymbol{\tau}_{i}=\boldsymbol{R}_{i} \boldsymbol{f}_{\theta}^{\prime}$
$\boldsymbol{h}_{i}=\boldsymbol{f}_{e}-\boldsymbol{k}_{e}$
The components of $\mathrm{f}_{r i}^{*}$ and $\mathrm{f}_{r i}$, for $r=1 \cdots n_{q}$, are then defined as:

$$
\begin{align*}
& \mathrm{f}_{r i}^{*}=\boldsymbol{J}_{v, r i} \cdot \boldsymbol{f}_{i}^{*}+\boldsymbol{J}_{\omega, r i} \cdot \boldsymbol{\tau}_{i}^{*}+\boldsymbol{J}_{e, r i} \cdot \boldsymbol{h}_{i}^{*}  \tag{18}\\
& \mathrm{f}_{r i}=\boldsymbol{J}_{v, r i} \cdot \boldsymbol{f}_{i}+\boldsymbol{J}_{\omega, r i} \cdot \boldsymbol{\tau}_{i}+\boldsymbol{J}_{e, r i} \cdot \boldsymbol{h}_{i} \tag{19}
\end{align*}
$$

with
$\boldsymbol{J}_{v, r i}=\frac{\partial \boldsymbol{v}_{O, i}}{\partial \dot{q}_{r}}, \quad \boldsymbol{J}_{\omega, r i}=\frac{\partial \boldsymbol{\omega}_{i}}{\partial \dot{q}_{r}}, \quad \boldsymbol{J}_{e, r i}=\frac{\partial \boldsymbol{q}_{e, i}}{\partial q_{r}}$
where $\boldsymbol{v}_{O, i}$ is the velocity of the body with respect to the inertial frame. The term $\boldsymbol{J}_{e, r i}$ consists of 0 and 1 because $\boldsymbol{q}_{e, i}$ is a subset of $\boldsymbol{q}$. Equation 18 and Equation 19, once evaluated for body $i$, are inserted in into Equation 5 to obtain the final equations of motion.

### 2.5 Nonlinear and linear equations of motion

The $n_{q}$ equations of motion given in Equation 4 are gathered into a vertical vector $\mathbf{e}$. They are recast into the form:
$\mathbf{e}(\boldsymbol{q}, \dot{\boldsymbol{q}}, \ddot{\boldsymbol{q}}, \boldsymbol{u}, t)=\mathbf{f}+\mathbf{f}^{*}=\boldsymbol{F}(\boldsymbol{q}, \dot{\boldsymbol{q}}, \boldsymbol{u}, t)-\boldsymbol{M}(\boldsymbol{q}) \ddot{\boldsymbol{q}}=\mathbf{0}$

$$
\begin{equation*}
\boldsymbol{M}(\boldsymbol{q}) \ddot{\boldsymbol{q}}=\boldsymbol{F}(\boldsymbol{q}, \dot{\boldsymbol{q}}, \boldsymbol{u}, t) \tag{22}
\end{equation*}
$$

where $\boldsymbol{M}=-\frac{\partial \mathrm{e}}{\partial \ddot{\boldsymbol{q}}}$ is the system mass matrix, and $\boldsymbol{F}$ is the forcing term vector, that vector-that is, the reminder remainder terms of the equation $(\boldsymbol{F}=\mathbf{e}+\boldsymbol{M} \ddot{\boldsymbol{q}})$. The vector $\boldsymbol{u}$ is introduced to represent the time-dependent inputs that are involved in the determination of the external loads. Both sides of the equations are also dependent on some parameters, but this dependency is omitted to shorten notations. The stiffness and damping matrices may be obtained by computing the Jacobian of the equations of motion with respect to $\boldsymbol{q}$ and $\dot{\boldsymbol{q}}_{2}$, respectively. The nonlinear equation given in Equation 22 is easily integrated numerically, for instance by recasting the system into a first-order system, or by using a dedicated second-order system time integrator.

In various applications, a linear time invariant approximation of the system is desired. Such approximation is obtained at an operating point, noted with the subscript 0 , which is a solution of the nonlinear equations of motion, viznamely:
$\mathbf{e}\left(\boldsymbol{q}_{0}, \dot{\boldsymbol{q}}_{0}, \ddot{\boldsymbol{q}}_{0}, \boldsymbol{u}_{0}, t\right)=\mathbf{0}$

The linearized equations about this operating point are obtained using a Taylor series expansion:
$\boldsymbol{M}_{0}\left(\boldsymbol{q}_{0}\right) \boldsymbol{\delta} \ddot{\boldsymbol{q}}+\boldsymbol{C}_{0}\left(\boldsymbol{q}_{0}, \dot{\boldsymbol{q}}_{0}, \boldsymbol{u}_{0}\right) \boldsymbol{\delta} \dot{\boldsymbol{q}}+\boldsymbol{K}_{0}\left(\boldsymbol{q}_{0}, \dot{\boldsymbol{q}}_{0}, \ddot{\boldsymbol{q}}_{0}, \boldsymbol{u}_{0}\right) \boldsymbol{\delta} \boldsymbol{q}=\boldsymbol{Q}_{0}\left(\boldsymbol{q}_{0}, \dot{\boldsymbol{q}}_{0}, \boldsymbol{u}_{0}\right) \boldsymbol{\delta} \boldsymbol{u}$
with
$\boldsymbol{M}_{0}=-\left.\frac{\partial \mathbf{e}}{\partial \ddot{\boldsymbol{q}}}\right|_{0}, \boldsymbol{C}_{0}=-\left.\frac{\partial \mathbf{e}}{\partial \dot{\boldsymbol{q}}}\right|_{0}, \boldsymbol{K}_{0}=-\left.\frac{\partial \mathbf{e}}{\partial \boldsymbol{q}}\right|_{0}, \boldsymbol{Q}_{0}=\left.\frac{\partial \mathbf{e}}{\partial \boldsymbol{u}}\right|_{0}$
where $\boldsymbol{M}_{0}, \boldsymbol{C}_{0}$, and $\boldsymbol{K}_{0}$ are the linear mass, damping, and stiffness matrices, $Q_{0}$ respectively; $\boldsymbol{Q}_{0} \boldsymbol{\delta u}$ is the linear forcing vector,$\left(Q_{0}\right.$ is the input matrix); $\delta$ indicate indicates a small perturbation of the quantities,; and $\left.\right|_{0}$ indicates that the expressions are evaluated at the operating point. Examples-In practical applications, linearization is done at an operating point where the acceleration is zero $\left(\ddot{\boldsymbol{q}}_{0}=0\right)$ and most velocities are also zero. Examples of applications of application of the linear equations of motion are $\div$ controller design, frequency domain analyses, and stability analyses. The symbolic system matrices also allow for the easy formulation of linear parameter-varying models used in many advanced control applications.

## 3 Implementation into a symbolic framework

In this section, we discuss a Python open-source symbolic calculation framework that implements the equations given in section 2. A Maxima implementation from the same authors is also available Geisler (2021).

The Python library YAMS (Yet Another Multibody Solver) started as a numerical tool published in previous work (Branlard, 2019). The library is now supplemented with a symbolic module so that both numerical and symbolic calculations can be achieved. The new implementation uses the Python symbolic calculation package SymPy (SymPy, 2021). We leveraged the features present in the subpackage "mechanics",", which contains all the tools necessary to compute kinematics: the definition of frames and points, and and the determination of positions, velocities, and accelerations. The subpackage also contains
an implementation of Kane's equations for rigid body bodies (i.e., subsection 2.3). We were also inspired by the package PyDy (Gede et al., 2013), which is a convenient tool to export the equations of motion to executable code -and directly visualize the bodies in 3D. The core of our work consisted in-of implementing a class to define flexible bodies (FlexibleBody) and the corresponding Kane's method for this class (subsection 2.4).

For the FlexibleBody class, we followed the formalism of Wallrapp (1994), and implemented Taylor expansions for all the terms defined in Appendix A, allowing the symbolic computation with shape functions of Taylor expansions to any order. The In practice, a zeroth- or first-order expansion is used. The use of Taylor expansions is presented in Appendix D3. The different Taylor coefficients may be kept as symbolic terms, or replaced early on by numerical values provided for instance by an SIDby a SID, for instance.

We structured the code into three layers-: 1) The low-level layer integrates seamlessly with SymPy and PyDy -by using the FlexibleBody class we provide. It is the layer which-that offers the highest level of granularity and control for the user, since arbitrary systems with various kinematic constraints can be implemented, at the cost of requiring more expertise. 2) The second-level automates the calculation of the kinematics -by introducing simple connections between rigid and flexible bodies. The connections may be rigid, with constant offsets and rotations, or dynamic. A connection from a flexible body to another body is assumed to occur at one extremity of the flexible body. Some knowledge of SymPy mechanics are is still required to use this layer. 3) The third level consists of template models such as generic land-based or offshore wind turbine models. Degrees of freedom are easily turned on and off for these conceptual models depending on the level of fidelity asked by the user $_{2}$ and generic external forces can be implemented or declared as external inputs. The-

The overall workflow for typical usage of the symbolic framework is illustrated in Figure $1_{\dot{\sim}}$ The symbolic framework


Figure 1. Typical workflow for the usage of the symbolic framework, going from numerical inputs and a conceptual model to numerical packages that can be used for various applications.
takes as input a conceptual model of the structure, which is assembled using one of the three layers previously described. The nonlinear and linear equations of motion can be exported to tatex LaTeX and Python-ready scripts for various applications (see subsection 5.1). As-Using the third layer, as little as three lines of code are required by the user to perform the full step from derivation of the equations, optional linearization, and exportation. To obtain numerical results from the exported Python code, the user needs to provide the arrays with the degrees of freedom values $\boldsymbol{q}$ and $\dot{\boldsymbol{q}}$, their initial conditions, a dictionary with inputs $(\boldsymbol{u})$ that are function functions of time, and a dictionary of parameters $(\boldsymbol{p})$ containing all the numerical constants such as mass, acceleration of gravity, etc. We provide tools to-and geometric parameters. We implemented various preprocessing tools in YAMS to facilitate the calculation of numerical parameters, typically from a set of OpenFAST input files or by using structural parameters defined by the users. YAMS contains tools to compute the flexible bodies parameters (mass matrix, stiffness matrix, shape integrals) using integrals over the shape functions or using a finite-element beam formulation. YAMS also contains tools to compute the rigid body inertia of different components of a wind turbine or the full system. Postprocessing tools are also included to readily time-integrate the generated model using numerical values (including initial values)frem a set of OpenFASF imput files.

The source code of YAMS is available on GitHub as a subpackage of the Wind Energy LIBrary, WELIB (Branlard, 2021). The repository contains tests and working examples, including the ones presented in section 4 . The finite element package of the repository can be used to generate the SID from beam models such as the ones used for blades and tower.

## 4 Wind energy applications

### 4.1 Approach

In this section, we present different wind energy applications of the symbolic framework. We focus on models with at least one flexible body because the rigid-body rigid body formulation of SymPy has been well verified (Gede et al., 2013). For each example, the equations of motion are given and their results are compared with OpenFAST (?) (Jonkman et al., 2021) simulations. This is readily achieved because our framework can export the equations of motion to Python functions, load input files from an OpenFAST model, and integrate the generated equations using the same conditions as defined in the OpenFAST input files. In this article, we do not focus on the modeling of the external loads, but we include them in the equations of motion. It is the responsibility of the user to define these functions, for instance through aero- or hydro-force models. For the verification results presented in this section, we only include the gravitational and inertial loading. In all examples, the NREL National Renewable Energy Laboratory (NREL) 5-MW reference wind turbine (Jonkman et al., 2009) is used. The examples below are provided on the GitHub repository where the YAMS package is provided (Branlard, 2021).

### 4.2 Notations

We adopt a system of notations where the first letter of a body is used to identify the parameters of that body. As an example, the tower is represented with the letter T , and the following body parameters are defined: $T$, origin; $M_{T}$, mass; $L_{T}$, length; ("flapwise"): $\Phi_{1}=\Phi \hat{x}$, where the hat notation indieates the $\Phi_{1}=\Phi e_{x}$, where $e_{x}$ is the unit vector in the $x$ direction. The given in Figure 2. We start by modeling the blade using a single shape function, assumed to be directed along the $x$ axis-axis


Figure 2. Sketch of a rotating blade ,-with the restoring centrifugal force. Points are indicated in green, degrees of freedom in blue, and loads in orange.
$230 \quad M_{e}=\int_{0}^{R} m(r) \Phi^{2}(r) d r$
$231 \quad K_{e}=\int_{0}^{R} E I_{y}(r)\left[\frac{d^{2} \Phi}{d r^{2}}(r)\right]^{2} d r$
The generalized force is obtained from Equation B3:
$f_{e}=\int_{0}^{R} p_{x}(r, t) \Phi(r) d r$

The important consideration for this model is the axial load, $N$. The main axial load at a radial station $r$ comes from the centrifugal force acting on all the points outboard of the current station:
$N(r)=\int_{r}^{R} m\left(r^{\prime}\right) \Omega^{2} r^{\prime} d r^{\prime}$
The geometric stiffness contribution of the axial load is obtained from Equation B5 as:
$K_{g}(\Omega)=\int_{0}^{R} N(r)\left[\frac{d \Phi}{d r}\right]^{2} d r=\Omega^{2} \int_{0}^{R} \int_{r}^{R} m\left(r^{\prime}\right) r^{\prime} d r^{\prime}\left[\frac{d \Phi}{d r}\right]^{2} d r$
The axial-geometric stiffness, $K_{g}$, is positive and increases with the square of the rotational speed. This restoring effect is referred to as "centrifugal stiffening".," The natural frequency of the blade will increase with the rotational speed as follows:
$\omega_{0}(\Omega)=\sqrt{\frac{\left(K_{e}+K_{g}(\Omega)\right)}{M_{e}}}=\sqrt{\omega_{0}^{2}(0)+\frac{K_{g}(\Omega)}{M_{e}}}=\sqrt{\omega_{0}^{2}(0)+k_{\Omega} \Omega^{2}}$
where $k_{\Omega}$ is referred to as the rise factor, or "rise factor" or "Southwell coefficient," and in our approximation, it is found to be constant: $k_{\Omega}=K_{g}(\Omega) / M_{e} / \Omega^{2}$. The coefficient provides the variation of the blade frequency with rotational speed, which is something that is observed on a Campbell diagram when performing stability analyses. In general, the mode shapes of the blade will also change as a function of the rotational speed, and different shape functions should preferably be used for simulations at different rotational speeds. The effect is fairly limited, and most OpenFAST practitioners only use one shape function corresponding to the value at rated rotational speed. Similarly, the Southwell coefficient is a function of the rotational speed, but the variation is negligible as long as the rotational speed is small compared to the natural frequency (e.g., $(\Omega / \omega)^{2} \lesssim 5$, ; see Bielawa (2006)), which is the case for wind energy applications.

The treatment for a shape function in the edgewise direction is similar, using $\Phi_{2}=\Phi_{2} \hat{\theta} \Phi_{2}=\Phi_{2} e_{\theta}$, where $e_{\theta}$ is the unit vector in the edgewise direction. In this case, the centrifugal force also has a component in the tangential directionequat to- $p_{\theta, \text { centri }}(r)=-\Omega^{2} u_{\theta}(r) d m(r)$, with $u_{\theta}=\Phi_{2} q$. This leads to a generalized force equal to $\int_{0}^{L} p_{\theta, \text { centri }} d r \Phi_{2}=\Omega^{2} M_{e} q$, or $\int_{a}^{L} p_{\theta, \text { sentri }} \Phi_{2} d r=-\Omega^{2} M_{e} q$, or, equivalently, to a stiffness term: $K_{\omega}=-\Omega^{2} M_{e}$. It can be verified that this generalized force corresponds to the contribution $O_{e, 11} \omega_{x}^{2}$, from $\boldsymbol{k}_{\omega, e}$, given in Equation A10. For an edgewise mode, the frequency therefore evolves as:
$\omega_{0}(\Omega)=\sqrt{\frac{\left(K_{e}+K_{g}(\Omega)+K_{\omega}(\Omega)\right)}{M_{e}}}=\sqrt{\omega_{0}^{2}(0)+\left(k_{\Omega}-1\right) \Omega^{2}}$
with $k_{\Omega}=K_{g}(\Omega) / M_{e} / \Omega^{2}$ and with $K_{g}$ computed using Equation 30.
We apply the method to the NREL 5-MW wind turbine using the blade properties and shape functions provided in the ElastoDyn input file. We order the degrees of freedom as $\div 1^{\text {st }}$ flap, $1^{\text {st }}$ edge, and $2^{\text {nd }}$ flap, assuming no coupling between the shape functions, so that each of them can be treated individually using the results from this section. The diagonal coefficients of the mass matrix $\operatorname{are} \operatorname{diag}\left(\boldsymbol{M}_{e}\right)=[9.5 e 3,1.5 e 4,5.7 e 3]$, and for the stiffness matrix they $\operatorname{are} \operatorname{diag}\left(\boldsymbol{K}_{e}\right)=[1.7 e 4,6.7 e 4,8.7 e 4]$, Figure 3. The simulations were run in vacuum (no gravity, no aerodynamics) and with a cone angle of 0 deg. Strong agreement


Figure 3. Variation of the natural frequencies of the NREL 5-MW turbine blade with rotational speed. Results from YAMS and OpenFAST, with mean relative error, $\epsilon$, are reported on the figure.
is found for the evolution of the different frequencies with the rotational speed. The stiffening is less pronounced for edgewise modes as a result of the softening introduced by $K_{\omega}$.

This section focused on the analysis of individual shape functions. In the general case, multiple shape functions are present and couplings might exist between them (due to the structural twist -or nonorthogonality of the shape functions, or if the shape functions have components in multiple directions such as $\left.\Phi_{1}=\Phi_{1 x} \hat{\boldsymbol{x}}+\Phi_{1 y} \hat{\boldsymbol{y}} \Phi_{1}=\Phi_{1 x} \boldsymbol{e}_{x}+\Phi_{1 \mu} \boldsymbol{e}_{y}\right)$. In such a case, the general developments of Appendix A and Appendix B should be used.

### 4.4 Two degrees of freedom model of a land-based or fixed-bottom turbine

We consider a system of three bodies: tower (or support structure), nacelle ${ }_{2}$ and rotor. The system represents an a land-based wind turbine or a fixed-bottom offshore wind turbine. A sketch of the system is given in Figure 4. The nacelle and rotor blades are rigid bodies, whereas the tower is flexible and represented by one shape function ${ }^{2}$ in the fore-aft direction, noted $\Phi_{1}=\Phi_{1} \hat{x} \Phi_{1}=\Phi_{1} e_{x}$. For hand calculations and as a first approximation, the first mode shape of a massless beam with a top mass may be used: $\Phi_{1}(z)=1-\cos (z \pi / L / 2)$. Increased accuracy is obtained when the shape function matches the actual first tower fore-aft bending mode, accounting for the effect of the rotor-nacelle mass and inertia. The degrees of freedom are $\boldsymbol{q}=(q, \psi)$, where $q$ is the generalized (elastic) coordinates in the fore-aft direction and $\psi$ is the azimuthal position. The slope of the tower shape function at the tower top is a key coupling parameter of the model, noted $\nu_{y}$. When the tower

[^1]

Figure 4. Model of a land-based or fixed-bottom wind turbine using one to three degrees of freedom (fore-aft and side-side flexibility of the support structure, and shaft rotation). Points are indicated in green, degrees of freedom in blue, and loads in orange.
deflects 1 m in the $x$ direction, the nacelle rotates by an angle $\nu_{y}$. The method assumes that the tower-top point remains along the $x$-axis, neglecting the so-called nonlinear geometric effect. However, nonlinear geometric effects can be included using geometric stiffening corrections (Branlard, 2019)(see Appendix C or Branlard (2019)). The aerodynamic thrust and torque are noted $f_{a}$, and $\tau_{a}$, aeting respectively and act at the rotor center (point $R$ ). The low-speed shaft generator torque is written as $\tau_{g}$. The distributed loads on the tower, $p_{x}$ (from aerodynamics and hydrodynamics), are projected against the shape function to obtain the generalized forces $\div f_{e}=\int_{0}^{L_{T}} p_{x}(z, t) \Phi_{1}(z) d z$. The moments of inertia of the rotor in its coordinates are $\left(J_{x, R}, J_{\oplus, R}, J_{\oplus, R}\right)$. We note that $M_{e}, K_{e}, D_{e}-M_{e} K_{e}$ and $D_{e}$ are the generalized mass, stiffness ${ }_{2}$ and damping, respectively, associated with a given shape function $\div M_{e}=\int_{0}^{L_{T}} m(z) \Phi_{1}^{2}(z) d z, K_{e}=\int_{0}^{L_{T}} E I(z)\left[\frac{d^{2} \Phi_{1}}{d z^{2}}(z)\right]^{2} d z, D_{e}=2 \zeta M_{e} \omega_{e}$, where $m(z)$ and $E I(z)$ are the mass per length and bending stiffness of the tower, respectively $y_{\imath}$ and $\omega_{e}$ and $\zeta$ are the frequency and damping ratio, respectively, associated with the shape function (assuming the shape function is close to approximates a mode shape). The geometric softening of the tower due to the tower-top mass ( $K_{g t}$ ) and its own weight ( $K_{g s}\left(K_{\alpha w}\right)$ is obtained using Equation B5, as $K_{g}=K_{g t}+K_{g s} K_{g}=K_{g t}+K_{q w}$, with :
$K_{g t}=-g \int_{0}^{L_{T}}\left(M_{R}+M_{N}\right)\left[\frac{d \Phi_{1}}{d z}(z)\right]^{2} d z$
$K_{\underline{g s} g w}=-g \int_{0}^{L_{T}}\left[\frac{d \Phi_{1}}{d z}(z)\right]^{2}\left[\int_{z}^{L_{T}} m\left(z^{\prime}\right) d z^{\prime}\right] d z$
The shape function frequency is obtained as:
$\omega_{e}=\sqrt{\left(K_{e}+K_{g}\right) / M_{e}}$

The application of the symbolic framework leads to the following equations of motion (rearranged for interpretability):
$\left[\begin{array}{cc}M_{q} & 0 \\ 0 & J_{x, R}\end{array}\right]\left[\begin{array}{c}\ddot{q} \\ \ddot{\psi}\end{array}\right]=\left[\begin{array}{c}f_{q} \\ \tau_{a}-\tau_{g}\end{array}\right]$
where:

$$
\begin{align*}
M_{q}= & M_{e}+M_{N}+M_{R}  \tag{37}\\
& +\left(J_{y N}+J_{\oplus, R}+M_{N}\left(x_{N G}^{2}+z_{N G}^{2}\right)+M_{R}\left(x_{N R}^{2}+z_{N R}^{2}\right)\right) \nu_{y}^{2}  \tag{38}\\
& +2\left[\left(M_{N} z_{N G}+M_{R} z_{N R}\right) \cos \left(\nu_{y} q\right)-\left(M_{N} x_{N G}+M_{R} x_{N R}\right) \sin \left(\nu_{y} q\right)\right] \nu_{y} \tag{39}
\end{align*}
$$

and

$$
\begin{align*}
f_{q} & =f_{e}-\left(K_{e}+K_{g}\right) q-D_{e} \dot{q}  \tag{40}\\
& +g \nu_{y}\left[\left(M_{N} x_{N G}+M_{R} x_{N R}\right) \cos \left(\nu_{y} q\right)+\left(M_{N} z_{N G}+M_{R} z_{N R}\right) \sin \left(\nu_{y} q\right)\right]  \tag{41}\\
& +\nu_{y}^{2} \dot{q}^{2}\left[\left(M_{N} x_{N G}+M_{R} x_{N R}\right) \cos \left(\nu_{y} q\right)+\left(M_{N} z_{N G}+M_{R} z_{N R}\right) \sin \left(\nu_{y} q\right)\right]  \tag{42}\\
& +f_{a} \nu_{y}\left(x_{N R} \sin \theta_{t}+z_{N R} \cos \theta_{t}\right)  \tag{43}\\
& +f_{a} \cos \left(\theta_{t}+\nu_{y} q\right) \tag{44}
\end{align*}
$$

Details on the derivations are given in Section ??Appendix E1. The mass matrix consists of three main contributions: Equation 37 represents the elastic mass and the rotor nacelle assembly (RNA) mass, Equation 38 is the generalized rotational inertia of the RNA, and Equation 39 is the inertial coupling between the tower bending and the rotation of the nacelle. The forcing terms are identified as follows: Equation 40 consists of the elastic load due to-resulting from the external forces on the tower, the elastic and geometric stiffness loads, and the damping load on the tower; Equation 41 is the gravitational load from the RNA $_{2}$ which will contribute to the stiffness of the system; Equation 42 is the centrifugal force of the RNA (" $M \omega^{2} r$ " with $\omega=\nu_{y} \dot{q}$; Equation 43 is the generalized torque from the aerodynamic thrust; and Equation 44 is the thrust contribution acting directly along the direction of the shape function degree of freedom (along $x$ ). The RNA center of mass plays an important part in the equations (see the terms $\left(M_{N} x_{N G}+M_{R} x_{N R}\right)$ and $\left.\left(M_{N} z_{N G}+M_{R} z_{N R}\right)\right)$.

The equations of motion given in Equation 36 can be used to perform time domain simulations of a wind turbine. It is noted that the two degrees of freedom are only coupled by the aerodynamic loads. The nonlinear model was used in previous work for time domain simulations and its linear version was used for state estimations (Branlard et al., 2020a, b). In this section, we apply the linearized form to compute the natural frequency of the turbine tower fore-aft mode. The linearized stiffness is here obtained by taking the gradient of the forcing with respect to $q$, and using a small angle approximation for $\nu_{y}$ to the second order:
$K_{q, l i n}=\left(K_{e}+K_{g}\right)-\nu_{y}^{2} g\left(M_{N} z_{N G}+M_{R} z_{N R}-f_{a} q \cos \theta_{t}\right)+\nu_{y} f_{a} \sin \theta_{t}$
For the NREL 5-MW reference turbine (Jonkman et al., 2009), the different numerical values are: $g=9.807 \mathrm{~m} \cdot \mathrm{~s}^{-2}, \theta_{t}=5$ $\mathrm{deg}, x_{N R}=-5.0 \mathrm{~m}, z_{N R}=2.4 \mathrm{~m}, L_{T}=87.6 \mathrm{~m}, z_{N G}=1.75 \mathrm{~m}, x_{N G}=1.9 \mathrm{~m}, M_{R}=1.1 e 5 \mathrm{~kg}, J_{x, R}=3.86 e 7 \mathrm{~kg} \mathrm{~m}^{2}$,


Figure 5. Properties of the NREL 5-MW turbine tower: mass per length ( $m$ ), bending stiffness ( $E I$ ), and shape function displacement ( $\Phi$ ), slope ( $d \Phi / d z$ ) and curvature $\left(d^{2} \Phi / d z^{2}\right)$.
tain the correct numerical values for the flexible tower, namely: $\nu_{y}=0.0185, M_{e}=5.4 e 4, K_{e}=1.91 e 6, K_{g}=-5.2 e 4-$ $1.0 e 4=-6.20 e 4, \omega_{e}=\sqrt{\left(K_{e}+K_{g}\right) / M_{e}}=5.85 \mathrm{rad} / \mathrm{s}$. These numerical values, with $q=0$, leads lead to: $M_{q}=4.375 e 5$ and $K_{q}=1.849 e 9$. The first fore-aft mode of the wind turbine has a natural frequency of $f=\sqrt{K_{q} / M_{q}}=0.3272 \mathrm{~Hz}$. This value was compared with results obtained using OpenFAST linearization. Both methods are in strong agreements-agreement, with differences only arising at the fifth decimal place.

### 4.5 Three-degrees-of-freedom model of a land-based or fixed-bottom turbine

We consider the same system as the one presented in subsection 4.4 , but the tower is now represented by one shape function in both the fore-aft and side-side directions, $\Phi_{1}=\Phi_{1} \hat{x}$ and $\Phi_{2}=\Phi_{2} \hat{y} \Phi_{1}=\Phi_{1} e_{x}$ and $\Phi_{2}=\Phi_{2} \boldsymbol{e}_{y}$. The degrees of freedom are $\boldsymbol{q}=\left(q_{1}, q_{2}, \psi\right)$, where $q_{1}$ and $q_{2}$ are the generalized (elastic) coordinates in the fore-aft and side-side directions, respectively, and $\psi$ is the rotor azimuth. A sketch of the system is given in Figure 4. and the rotational speed is 5 rpm . We report the mean relative error, $\epsilon$, and the coefficient of determination, $R_{2}^{2}$, on the figure.


Figure 6. Free decay results for the land-based/fixed bettom fixed-bottom model using both the symbolic framework (YAMS) and OpenFAST. From top to bottom: tower fore-aft bending, tower side side side-side bending, and shaft rotational speed.

We observe that our model is in strong agreement with the OpenFAST simulation. The differences in the second tower degree of freedom are attributed to $\div$ ) the handling of the small-angle approximation, which is different in OpenFAST (using the closest orthonormal matrix,-; Jonkman (2009)) and in our formulation (two successive rotations, linearized); 2) the nonlinear with a selected fore-aft shape function. A sketch of the model is given in Figure 7. The notations are similar to the ones


Figure 7. Model of a floating wind turbine using 3-three degrees of freedom. Points are indicated in green, degrees of freedom in blue, and loads in orange.
presented in subsection 4.5. Lumped hydrodynamic loads at the floater center of mass are now added. The model can also be used for a combined tower and floater that is flexible, simply by setting the mass of the floater to zero and including the hydrodynamic loading into the loading $p_{x}$. The equations of motion are given in Appendix E3. The equations were simplified using a first-order small-angle approximation of $\theta_{t}$ and $\phi_{y}$, and a second-order approximation for $\nu_{y}$.

We performed a numerical simulation of the model generated by YAMS and compared it with OpenFAST -for a case with gravitational loads only, starting with $x=0 \mathrm{~m}, \phi=2 \mathrm{deg}$, and $q_{T}=1 \mathrm{~m}$. The results are presented in Figure 8 . We observe again that the results from the two models correlate to a high degree.

We also compared the linearized version of both models. The symbolic framework can generate the linearized mass, stiffness, $\boldsymbol{z}_{2}$ and damping matrices, as described in subsection 2.5 . The matrices are then combined into a state matrix and compared with


Figure 8. Free decay Free-decay results for the floating wind turbine model using YAMS and OpenFAST. From top to bottom: surge, pitch, and tower fore-aft bending.
the state matrices written by the OpenFAST linearization feature. The eigenvalue analysis of the YAMS state matrix returned a pitch and fore-aft frequencies of 0.099 Hz and $0.799 \mathrm{~Hz}_{2}$ respectively, whereas OpenFAST returned 0.095 Hz and 0.795 Hz . The $4 \%$ error in the pitch frequency appears reasonable in view of the approximations used.

## 5 Discussions

### 5.1 Applications and advantages of the method

The implementation of the symbolic YAMS library was originally motivated by the need to obtain a simple linearized model of a floating wind turbine for frequency domain simulations. The There are multiple potential applications of the frameworkare yet multiple:

- The generated equations can be used in time domain simulation tools. The equations can be readily exported to different programming languages (C, FORTRAN ${ }_{2}$ or Python) providing computationally efficient tools, particularly because the method generates compact, and minimal equations. This is in contrast to most other multibody codes, in which many terms are calculated as matrix equations and through successive function calls. Further, the symbolic framework allows us to generate optimized code, in which common terms and factors are computed once and stored in temporary variables for reuse in the different expressions. In our examples, time domain simulations were observed to be 2 orders of magnitude faster when using the automatically generated code in Python compared to OpenFAST simulations that rely on a compiled language. Using such a framework can be considered in the future to replace the existing ElastoDyn module of OpenFAST. It can also be applied to unusual configurations such as multirotor or vertical axis-vertical-axis
turbine concepts. Dedicated code can be generated for specific applications for increased performance. For instance, implicit integrators with iterative Newton-Raphson-like solvers benefit from the possibility generate-of generating exact and efficient Jacobians along with the equations of motion.
- The generation of linearized models has a wide range of applications, such as $\div$ linear time domain simulations, controller design and tuning, frequency domain analyses, stability analyses, state observers, or digital twins. The symbolic approach is severalfold faster than alternative approaches because it can be evaluated for all operating points at once, whereas other methods (e.g. 2 OpenFAST, HAWCStab2) require multiple linearization calls. Linearization-
- Analytical linearization with respect to parameters ean also be performed, making the method even more appealing, is directly obtained using our tool, which can be used for sensitivity analyses, parameter studies, optimizations, integrated design approaches, and controls co-design (e.g.for controller designs based on linear parameter varying approaches such as linear matrix-inequality based designs(Pöschke et al., 2020)., using methods such as linear-matrix-inequality-based designs) (Pöschke et al., 2020). Nonanalytical approaches require numerous linearizations and evaluations at various operating points (Jonkman et al., 2022).
- In addition to the nonlinear or linear equations of motion in minimal coordinates, the equations for the constraint forces or any auxiliary kinematic variable can also be generated efficiently by inserting unknown virtual displacements in the equations (see Appendix D5 for an alternative approach). The position of all bodies in local or global coordinates can be recovered from the minimal coordinates and, in combination with the flexible code generation, be used to output data (e.g., for 3D animations of the turbine).
- Analytical gradients of the equations can be computed and used in optimizations, nonlinear model predictive control, or moving horizon estimation. External loads that cannot be expressed analytically can be defined as generic functions of the structural degrees of freedom, inputs, and parameters. After the code generation, the user can link a numerical implementation of the function and its numerical gradients to be able to use a mix of analytical and numerical gradients.


### 5.2 Advantage of using a symbolic framework

Most advantages have already been diseussed in, namely: the wide range of applieations and the potential gain in eomputational time. Additionally, the method can provide-

- Another advantage of the presented method is the possibility to quickly generate models with different levels of detail, ensuring consistency between the different levels of fidelity. This is in contrast to other more heuristic modeling approaches in which parameters often have to be retuned for each added degree of freedom.
- The method provides useful insights and can be used as an educational tool: simple models of a system with few degrees of freedom can readily be obtained, studied, and compared to hand-based calculation.


### 5.2 Advanced consideration

Section 2 addressed the systematic derivation of the equations of motion for an assembly of rigid or flexible bodies. Some advanced aspects of the method are discussed here:

- The expression of the displacement field $\boldsymbol{u}$ in terms of a superposition of shape function is typically done using a first or second-order expansion. We discuss these formulations in. Our framework supports both approachesThe different terms involved in the equations of motion of flexible bodies can be decomposed using shape integrals (see Appendix D3). Our framework readily supports this optional decomposition: it is the responsibility of the user to provide the terms and values of the expansion when numerical evaluation is to occur. The advantage of the second-order expansion is that some geometric nonlinearities are directly accounted for by the method, whereas these nonlinearities need to be introduced "mantally" in the first-order expansion approach, as done in Branlard (2019).
- The definition of geometric stiffening requires attention in the general case. It is accounted for by the term $\boldsymbol{k}_{\sigma}$, presented in Appendix A. A general account of the effect for an arbitrary geometry can be found in Wallrapp and Sehwertassek (1991) and simplified expressions are given for beams in. We discuss geometric stiffening in more detail in Appendix C .
- The treatment of external loads was not addressed in detail in this article because the loads are application-specific (aerodynamics, hydrodynamics, etc.). The framework can accept external loads as arbitrary functions of multiple variables ; or as analytical expressions. In the former case, the user will have to provide an implementation of the function during the execution.
- Even though the equations of motion are void of constraint forces, the values of these forces can be recovered. They can be expressed as functions of the external forces and the states of the system. It is not necessary to compute them by iteratively solving constraint equations.
- The framework can easily include rheonomous constraints-for instance, for the pitch angle—without having to supply a dedicated torque. Pitch speed and accelerations can be directly introduced into the mechanical system if they are provided e.g. by a generic second-order pitch actuator model.


### 5.3 Limitations

In spite of the advantages listed in subsection 5.1, the symbolic procedure presented in this work has some potential limitations. We are identifying two in this section. First, constraints and closed loops have currently not been added to the framework. The SymPy mechanics package supports additional constraint equations within the Kanemethod. It is therefore hoped that this eurrentKane's method. We therefore hope that this limitation can be lifted in the future. Second, large problems may challenge a symbolic calculation package: memory impact, calculation time, simplification times, and size of expressions may become significant. Some of these issues may be alleviated by introducing intermediate variables that are only substituted for in the numerical implementation or by using a recursive formulation of the solution procedure (Branlard, 2019).

## 6 Conclusions

We presented a symbolic framework to obtain the linear and nonlinear equations of motion of a multibody system made of rigid bodies, flexible bodies, and kinematic joints. Our approach is based on Kane's method and a nonlinear shape function representation of flexible bodies. We provided different wind energy examples and verified the results against OpenFAST simulations. The framework can readily provide models suitable to a wide range of applications - with competitive computational times. The framework is open source, and the examples presented are available in the repository. Future work will focus on applying the framework to dedicated research projects, with more complex systems, and potentially extend the framework to account for closed-loop systems and arbitrary constraints.

Author contributions. Both authors exchanged over the last two years about the implementation of such a framework and its application to wind energy. EB wrote a Python implementation and JG wrote a Maxima implementation. EB wrote the main corpus of the article, with feedback and contributions from JG.

Competing interests. No competing interests are present.

Code availability. A Zenodo link will be created for https://github.com/ebranlard/yams. The examples given in this articles are found in the folder welib/yams / papers of the repository.

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In this section, we detail the equations of motion of a flexible body. The reader is referred to the following references for a complete treatment of the equations of motion: Shabana (2013), Schwertassek and Wallrapp (1999), and Wallrapp (1994). The subscript $i$, indicating the body index, is dropped. All quantities (vectors and matrices) are expressed in the body frame of reference; therefore, the prime notation is also dropped in this section. The number of flexible shape functions associated with the body is $n_{e}$, the flexible degrees of freedom are $\boldsymbol{q}_{e}$, and the shape functions are gathered into a matrix $\boldsymbol{\Phi}$ of size $\left(3 \times n_{e}\right)$. Primes are used to indieate that quantities are expressed in the body frame of reference. The equations of motion, given in Equation 11, are repeated below:

$$
\left[\begin{array}{ccc}
\boldsymbol{M}_{x x} & \boldsymbol{M}_{x \theta} & \boldsymbol{M}_{x e}  \tag{A1}\\
& \boldsymbol{M}_{\theta \theta} & \boldsymbol{M}_{\theta e} \\
\mathrm{sym} . & & \boldsymbol{M}_{e e}
\end{array}\right]\left[\begin{array}{c}
\boldsymbol{a}_{i} \\
\dot{\boldsymbol{\omega}} i \\
\ddot{\boldsymbol{q}}_{e}
\end{array}\right]+\left[\begin{array}{c}
\boldsymbol{k}_{\omega, x} \\
\boldsymbol{k}_{\omega, \theta} \\
\boldsymbol{k}_{\omega, e}
\end{array}\right]+\left[\begin{array}{c}
0 \\
0 \\
\boldsymbol{k}_{e}
\end{array}\right]=\left[\begin{array}{c}
\boldsymbol{f}_{x} \\
\boldsymbol{f}_{\theta} \\
\boldsymbol{f}_{e}
\end{array}\right]
$$

The different terms of the mass matrix are obtained as follows:
$\boldsymbol{M}_{x x}=\int \boldsymbol{I}_{3} \mathrm{~d} m=M \boldsymbol{I}_{3} \quad(3 \times 3)$
$\boldsymbol{M}_{x \theta}=-\int \tilde{\boldsymbol{s}}_{P} \mathrm{~d} m=-M \tilde{\boldsymbol{s}}_{C M} \quad(3 \times 3)$
$\boldsymbol{M}_{\theta \theta}=-\int \tilde{\boldsymbol{s}}_{P} \tilde{\boldsymbol{s}}_{P} \mathrm{~d} m=\boldsymbol{J} \quad(3 \times 3)$
$\boldsymbol{M}_{\theta e}=\int \tilde{\boldsymbol{s}}_{P} \boldsymbol{\Phi} \mathrm{~d} m=\boldsymbol{C}_{r}^{T} \quad\left(3 \times n_{e}\right)$
$\boldsymbol{M}_{x e}=\int \boldsymbol{\Phi} \mathrm{d} m=\boldsymbol{C}_{t}^{T} \quad\left(3 \times n_{e}\right)$
$\boldsymbol{M}_{e e}=\int \boldsymbol{\Phi}^{T} \boldsymbol{\Phi} \mathrm{~d} m \quad\left(n_{e} \times n_{e}\right)$
The integrals are understeod as volume integrals over the volume of the body (for beams, they reduce to line integrals). The notation [~] represents the skew symmetric matrix. $M$ is the mass of the body. The vector $\boldsymbol{s}_{C M}$ is the vector from the origin of the body to undeflected center or mass $(\mathrm{CM})$ of the body. The notations $\boldsymbol{C}_{t}\left(n_{e} \times 3\right)$ and $\boldsymbol{C}_{r}\left(n_{e} \times 3\right)$ are introduced to match Wallrapp's notations. The vector $\boldsymbol{s}_{P}$ is the vector from the origin of the body to a deflected point of the body of elementary mass $\mathrm{d} m$. The undeflected position of this point is written as $\boldsymbol{s}_{P_{0}}$ and the displacement field $\boldsymbol{u}$, such that: $\boldsymbol{s}_{P}=\boldsymbol{s}_{P_{0}}+\boldsymbol{u}$. For a first-order expansion of Typically, the displacement field, is given by $\boldsymbol{u}=\boldsymbol{\Phi} \boldsymbol{q}_{e}$. Second-order expansions need the introduction of an additional notation: $\boldsymbol{u}=\boldsymbol{\Phi}_{u}\left(\boldsymbol{q}_{e}\right) \boldsymbol{q}_{e}$ (see and Wallrapp (1994) ). $\boldsymbol{\sim}$ but a higher-order expansion can also be introduced (see Wallrapp (1994) and Appendix D4). Wallrapp also includes the elementary mass moment of inertia, which results in additional terms in the integrals (see Wallrapp (1994)). Such contributions are relevantfor instance-, for instance, when considering the torsion of a beam (see Branlard (2019)). The block matrices $\boldsymbol{M}_{x x}, \boldsymbol{M}_{x e}$, and $\boldsymbol{M}_{e e}$ do not depend on the deformation of the body and are hence therefore constant. The other terms are functions of $\boldsymbol{q}_{e}$. They may be expressed as linear eombination combinations of constant integrals. These integrals are ustally referred to as shape integrals (Shabana (2013))or Taylor series coefficients (Wallrapp (1994))(see Appendix D3).

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The quadratic velocity terms, $\boldsymbol{k}_{\omega}$, are given as:
$\boldsymbol{k}_{\omega, x}=2 \tilde{\boldsymbol{\omega}} \boldsymbol{C}_{t}^{T} \dot{\boldsymbol{q}}_{e}+M \tilde{\boldsymbol{\omega}} \tilde{\boldsymbol{\omega}} \boldsymbol{s}_{C M} \quad(3 \times 1)$
(A8)
$\boldsymbol{k}_{\omega, \theta}=\tilde{\boldsymbol{\omega}} \boldsymbol{M}_{\theta \theta} \boldsymbol{\omega}+\left[\sum_{j=1 . . n_{e}} \boldsymbol{G}_{r, j} \dot{q}_{e, j}\right] \boldsymbol{\omega} \quad(3 \times 1)$
$\boldsymbol{k}_{\omega, e}=\left[\boldsymbol{\omega}^{T} \boldsymbol{O}_{e, j} \boldsymbol{\omega}\right]_{j=1 . . n_{e}}+\left[\sum_{j=1 \ldots n_{e}} \boldsymbol{G}_{e, j} \dot{q}_{e, j}\right] \boldsymbol{\omega} \quad\left(n_{e} \times 1\right)$
where
$\boldsymbol{G}_{r, j}=-2 \int \tilde{\boldsymbol{s}}_{P} \tilde{\boldsymbol{\Phi}}_{j} \mathrm{~d} m \quad(3 \times 3)$
$\boldsymbol{O}_{e, j}=\int \tilde{\boldsymbol{\Phi}}_{j} \tilde{\boldsymbol{s}}_{P} \mathrm{~d} m=-\frac{1}{2} \boldsymbol{G}_{r, j}^{T} \quad(3 \times 3)$
$\boldsymbol{G}_{e, j}=-2 \int \boldsymbol{\Phi}^{T} \tilde{\boldsymbol{\Phi}}_{j} \mathrm{~d} m \quad\left(n_{e} \times 3\right)$
The first term of Equation A10 is obtained by vertically stacking the contribution of each shape function. In the SID-standard input data format, this term is reshaped as the product $\boldsymbol{O}_{e} \boldsymbol{\Omega}$, where:
$\boldsymbol{O}_{e}=\left[O_{e, j, 11}, O_{e, j, 22}, O_{e, j, 33}, O_{e, j, 12}+O_{e, j, 21}, O_{e, j, 23}+O_{e, j, 32}, O_{e, j, 13}+O_{e, j, 31}\right]_{j=1 . . n_{e}} \quad\left(n_{e} \times 6\right)$ $\boldsymbol{\Omega}=\left[\omega_{x}^{2}, \omega_{y}^{2}, \omega_{z}^{2}, \omega_{x} \omega_{y}, \omega_{y} \omega_{z}, \omega_{x} \omega_{z}\right] \quad(6 \times 1)$

The body elastic forces are given by:
$\boldsymbol{k}_{e}=\boldsymbol{k}_{\sigma}+\boldsymbol{K}_{e} \boldsymbol{q}_{e}+\boldsymbol{D}_{e} \dot{\boldsymbol{q}}_{e}$
where $\boldsymbol{K}_{e}$ and $\boldsymbol{D}_{e}$ are the elastic stiffness and damping matrices, and $\boldsymbol{k}_{\sigma}$ represents stress stiffening terms geometric stiffening terms (see Appendix C). The elastic damping forces are often given as stiffness proportional damping. For more details, see Wallrapp (1994), and for more examples of with elastic beams, see Branlard (2019). The external loads can be assumed to consist of distributed volume forces, $\boldsymbol{p}$ (in practice they are mortly primarily surface forces or line forces), and a gravitational acceleration field $\boldsymbol{\sim}$ $\boldsymbol{g}$. The components of the external loads in Equation A1 are then obtained by integration over the whole body:
$\boldsymbol{f}_{x}=\int \boldsymbol{p} d V+\boldsymbol{M}_{x x} \boldsymbol{g} \quad(3 \times 1)$
$\boldsymbol{f}_{\theta}=\int \boldsymbol{s}_{P} \times \boldsymbol{p} d V+\boldsymbol{M}_{\theta x} \boldsymbol{g} \quad(3 \times 1)$
$\boldsymbol{f}_{e}=\int \boldsymbol{\Phi}^{T} \boldsymbol{p} d V+\boldsymbol{M}_{e x} \boldsymbol{g} \quad\left(n_{e} \times 1\right)$

## Appendix B: Application of the shape function approach to an isolated beam

In this section, we illustrate how the elastic equations of Appendix A can be applied to an isolated beam. Examples of applications are further given in subsection 4.3 and subsection 4.4. We consider a beam directed along the $z$-axis and bending in the $x$ and $y$ directiondirections. Expressions are written in the coordinate system of the beam and primes are dropped in this section. The beam properties are the following: length, $L$, mass per length, $m$, and bending stiffness $E I_{x}$ and $E I_{y}$. We assume that the displacement field is such that the shape functions are
functions of $z$ only: $\boldsymbol{u}(z, t)=\sum_{i=1}^{n_{e}} \boldsymbol{\Phi}_{i}(z) q_{e, i}(t)$. We also assume that the shape functions satisfy at least the geometric boundary conditions. The kinetic energy of the beam is $T=\frac{1}{2} \int_{0}^{L} m \dot{u}^{2} d z=\frac{1}{2} \sum_{i} \sum_{j} M_{e, i j} \dot{q}_{e, j} \dot{q}_{e, i}$. where $M_{e, i j}$ is (see Equation A7):
$M_{e, i j}=\int_{0}^{L} m(z) \boldsymbol{\Phi}_{i}(z) \cdot \boldsymbol{\Phi}_{j}(z) d z, \quad i, j=1, \ldots n_{e}$
Equation B1 involves a scalar product of the shape functions at each spanwise position. Integrals over the moment of inertia can be used to account for torsion (see Branlard (2019)). The potential energy (strain energy) of the beam, is obtained as $V=\frac{1}{2} \sum_{i} \sum_{j} K_{e, i j} q_{e, i} q_{e, j}$, where $K_{e, i j}$ are the element elements of the stiffness matrix, which, under the assumption of small deformations, are given by:
$K_{e, i j}=\int_{0}^{L}\left[E I_{y} \frac{d^{2} \Phi_{i, x}}{d z^{2}} \frac{d^{2} \Phi_{j, x}}{d z^{2}}+E I_{x} \frac{d^{2} \Phi_{i, y}}{d z^{2}} \frac{d^{2} \Phi_{j, y}}{d z^{2}}\right] d z, \quad i, j=1, \ldots n_{e}$
Elongation and torsional strains ( $E A$ and $G K_{t}$ ) can similarly be added to the strain energy and the stiffness matrix if longitudinal and torsional displacement fields are included in the shape functions. The external loads on the beam are assumed to consist of a distributed force vector, $\boldsymbol{p}(z)$. The virtual work done by the force $\boldsymbol{p}$ for each virtual displacement $\delta q_{e, i}$, provides the generalized force as (see Equation A17):
$f_{e, i}=\int_{0}^{L} \boldsymbol{\Phi}_{i} \cdot \boldsymbol{p} d z$
The equations of motion of the isolated beam and then written in matricial matrix form as:
$\boldsymbol{M}_{e} \ddot{\boldsymbol{q}}_{e}+\boldsymbol{D}_{e} \dot{\boldsymbol{q}}_{e}+\boldsymbol{K}_{e} \boldsymbol{q}_{e}=\boldsymbol{f}_{e}$
where $\boldsymbol{q}_{e}=\left[q_{e, 1}, \cdots, q_{e, n}\right]$. Damping is typically added a posteriori to the equations, where the Rayleigh damping assumption is often used: $\boldsymbol{D}_{e}=\alpha \boldsymbol{M}_{e}+\beta \boldsymbol{K}_{e}$ (stiffness proportional damping implies $\alpha=0$ ). If the shape functions are mode shapes, then the shape functions are orthogonal, the mass and stiffness matrices are diagonal, and the stiffness values would be $K_{e, i i}=\omega_{e, i}^{2} M_{e, i i}$, with $\omega_{e, i}=\sqrt{K_{e, i i} / M_{e, i i}}$ the eigenfrequency of the beam mode $i$. The modal damping is then given by $D_{e, i i}=2 \zeta_{i} M_{e, i i} \omega_{e, i}$, where $\zeta_{i}$ is the damping ratio associated with mode $i$.

If the beam is loaded axially by a force $N(z)$, then this force produces a distributed load in the transverse direction equal to $\boldsymbol{n}=$ $\frac{\partial}{\partial z}\left[N(z) \frac{\partial u}{\partial z}\right]$, with components in the $y$ and $z$ directions (see Branlard (2019)). The generalized force associated with this loading is then $\div Q_{N, i}=\int_{0}^{L} \boldsymbol{\Phi}_{i} \cdot \boldsymbol{n} d z$. Inserting the expression of $\boldsymbol{n}$ and $\boldsymbol{u}$, the generalized force has the form of a stiffness term: $Q_{N, i}=-\sum_{j} K_{N, i j} q_{j}$ with
$K_{N, i j}=-\int_{0}^{L} \boldsymbol{\Phi}_{i} \cdot \frac{d}{d z}\left[N(z) \frac{d \mathbf{\Phi}_{j}}{d z}\right] d z=\int_{0}^{L} N(z) \frac{d \boldsymbol{\Phi}_{i}}{d z} \cdot \frac{d \mathbf{\Phi}_{j}}{d z}-\left[N(x) \boldsymbol{\Phi}_{i} \cdot \frac{d \mathbf{\Phi}_{j}}{d z}\right]_{0}^{L}$
and where integration by parts was used to obtain the second equality. Examples of applications are given in subsection 4.3 and -subsection 4.4. The fact that an axial load leads to a stiffness term is referred to as "geometric stiffness," which is the topic of Appendix C .

## Appendix C: Geometric stiffness

## C1 General treatment

$589 \quad K_{g t, z, i j}=\int_{0}^{L} N(z) \frac{d \boldsymbol{\Phi}_{\boldsymbol{i}}}{d z} \cdot \frac{d \boldsymbol{\Phi}_{\boldsymbol{j}}}{d z} d z, \quad N(z)=\int_{z}^{L} m(z) d z$
$611 \boldsymbol{J}=\left[\begin{array}{l}\boldsymbol{J}_{v} \\ \boldsymbol{J}_{\omega} \\ \boldsymbol{J}_{e}\end{array}\right]$
We write $z_{k}$ the coordinate of node $k$ along the beam. The $i j$-component of the matrix $K_{d F}^{k}$ is obtained as:
$K_{g F, z, i j}^{k}=\int_{0}^{L} N(z) \frac{d \mathbf{\Phi}_{\boldsymbol{i}}}{d z} \cdot \frac{d \mathbf{\Phi}_{\boldsymbol{j}}}{d z} d z, \quad N(z)=1$ if $z<z_{k}, 0$ otherwise
The $i j$-component of the matrix $\boldsymbol{K}_{g \omega, \alpha \beta}$ is obtained by considering unit centrifugal loads generated using independent rotations around the unit vectors $\boldsymbol{e}_{x}, \boldsymbol{e}_{y}$, and $\boldsymbol{e}_{z}$ :
$K_{g \omega, \alpha \beta, i j}=\int_{0}^{L}-\boldsymbol{e}_{z} \cdot\left(\tilde{\boldsymbol{e}}_{\alpha} \tilde{\boldsymbol{e}}_{\beta} \boldsymbol{N}(z)\right) \frac{d \boldsymbol{\Phi}_{\boldsymbol{i}}}{d z} \cdot \frac{d \boldsymbol{\Phi}_{\boldsymbol{j}}}{d z} d z, \quad \boldsymbol{N}(z)=\int_{z}^{L} m(z) \boldsymbol{s}_{P_{0}} d z$
Similarly, the $i j$-component of the matrix $\boldsymbol{K}_{g r, \alpha}$ is:
$K_{g r, \alpha, i j}=\int^{L}-\boldsymbol{e}_{z} \cdot\left(\tilde{\boldsymbol{e}}_{\alpha} \boldsymbol{N}(z)\right) \frac{d \boldsymbol{\Phi}_{\boldsymbol{i}}}{d z} \cdot \frac{d \boldsymbol{\Phi}_{\boldsymbol{j}}}{d z} d z, \quad \boldsymbol{N}(z)=\int^{L} m(z) \boldsymbol{s}_{P_{0}} d z$

## C3 Integration into the equations of motion

The term $\boldsymbol{k}_{\boldsymbol{q}}=\boldsymbol{K}_{g} \boldsymbol{g}_{e}$ appears on the third block-row of the equations of motion of the flexible body (Equation A1). Because of the linearity with respect to the acceleration, rotational velocities, and forces, the different contributions can optionally be incorporated into the third block-row of the mass matrix $\left(\boldsymbol{M}_{e *}\right)$, the term $\boldsymbol{k}_{\omega}$,e and the term $\boldsymbol{f}_{\boldsymbol{e}}$, respectively. For instance, the term $\sum_{a} a_{\alpha} \boldsymbol{K}_{q t, \alpha} \boldsymbol{q}_{\varepsilon}$ can be reorganized as $\left[\boldsymbol{K}_{g t}\right] \boldsymbol{q}_{e} \cdot \boldsymbol{a}$ (using loose notations); therefore, the mass matrix can be updated such that $\boldsymbol{M}_{x e}$ becomes $\boldsymbol{M}_{x e}+\left[\boldsymbol{K}_{g t}\right] \boldsymbol{q}_{e}$. When a Taylor expansion is used, such integration is easily implemented as a first-order term (see Appendix D3).

## Appendix D: Alternative formulations

Different formulations of flexible multibody dynamics using shape functions are found in the literature. Some of the alternatives are briefly discussed in this section.

## D1 Jacobian and velocity transformation matrix

In Equation 7, the Jacobian terms $\boldsymbol{J}$ and the virtual work are expressed in vector form. In such form, there is no need to precise-state in which coordinate system the different vectors are expressed. This is convenient to reduce the size of the expressions when using symbolic calculations. In a numerical framework, the vector will have to be expressed in a common frame. When such an approach is used (see, e.g., Lemmer (2018); Branlard (2019)), the Jacobians are sometimes stacked into a matricial matrix form:
(D1)

Some implementation choices are needed depending if these matrices are expressed in the global frame or a body frame. The Jacobian matrices are referred to as "velocity transformation matrix," and the link between formulations in global and local coordinates is given in Branlard (2019). In the same reference, recursive relationships are given for tree-like assembly of bodies ,-to help express the Jacobian
matrices of each body recursively, based on the matrices of the parent body. It is also noted that the quadratic velocity terms, $\boldsymbol{k}_{\omega}$, can be obtained using the time derivative of the Jacobian matrix.

## D2 Rotations and torsion

In this article, we have not explicitly written the rotational impact of the elaborated on the change of orientation introduced by shape functions. In most applications, bodies are connected at their extremities and the deflection slope at a body extremity will induce a rotation of the subsequent body - (e.g., tilting and rolling of the nacelle at the tower top). The deflection slope can be obtained form the knowledge of the shape functions. This is readily accounted for by introducing a time-varying rotation matrix between bodies, and this is the approach used in our symbolic framework. A formalism of rotations of bodies connected at their extremities is given in Branlard (2019). A more general formulation, introducing shape function rotations $\Psi$, is given in (Wallrapp, 1994; Schwertassek and Wallrapp, 1999; Lemmer, 2018). In such


## D3 Shape integrals and Taylor expansion

The order of expansion of the displacement field leads to alternative formulations. In Shabana (2013) and Branlard (2019) a first-order expansion is used: $\boldsymbol{u}=\sum_{j}\left[\Phi_{j}^{0}\right] q_{e, j}$. In the work of Wallrapp a second-order expansion is used: $\boldsymbol{u}=\sum_{j}\left[\Phi_{j}^{0}+\frac{1}{2} \sum_{k} \Phi_{j k}^{1} q_{e, k}\right] q_{e, j}$. In beth formulations, the equations of motion given in The results presented in Appendix A tead to shape-integral expansions of the following form:
$\underline{\boldsymbol{T}=\boldsymbol{T}^{0}+\sum_{j=1 . . n_{e}} \boldsymbol{T}_{j}^{1} q_{e, j}}$
where $T$ is a dummy variable standingforconsist of integrals over the displaced points of the structure, $s_{P}=s_{P a}+u$, where the displacement field is $u=\Phi \boldsymbol{q}_{e}$. The undeflected position of the structure $\left(s_{P_{Q}}\right)$ is constant, and the shape functions are known at the initialization; the only time-varying terms are the degrees of freedom $\boldsymbol{q}_{e}$. Therefore, the integrals can be precomputed by decomposing them into a constant part and a part that is linear with respect to the degrees of freedom $\boldsymbol{q}_{\boldsymbol{e}}$. The precomputed integrals are referred to as "shape integrals." For a given term $\boldsymbol{T}$ (standing, for instance, for $\boldsymbol{M}_{\theta, \theta}, \boldsymbol{C}_{t}, \boldsymbol{C}_{r}, \boldsymbol{G}_{r}, \boldsymbol{G}_{e}$, or $\boldsymbol{O}_{e}$. The " 0 " and " 1 " terms), the shape integral expansion is:
$\boldsymbol{T}\left(\boldsymbol{q}_{e}\right)=\boldsymbol{T}^{0}+\sum_{j=1 . . n_{e}} \boldsymbol{T}_{j}^{1} q_{e, j}$
If $\boldsymbol{T}$ is an array, $\boldsymbol{T}^{0}$ and $\boldsymbol{T}_{\dot{\alpha}}^{1}$ have the same shape as $\boldsymbol{T}$. As an example, the application of the shape integral expansion to the term $\boldsymbol{M}_{x \Theta}$ (see Equation A3) gives:
$\boldsymbol{M}_{x \theta}=-\int \tilde{\boldsymbol{s}}_{P} \mathrm{~d} m=\boldsymbol{M}_{x \theta}^{0}+\sum_{j=1 \ldots n_{e}} \boldsymbol{M}_{x \theta, j}^{1} q_{e, j}^{\sim}$
with
$\boldsymbol{M}_{x \theta}^{0}=-\int \tilde{\boldsymbol{s}}_{P_{0}} \mathrm{~d} m, \quad \boldsymbol{M}_{x \theta, j}^{1}=-\int \tilde{\boldsymbol{\Phi}}_{j} \mathrm{~d} m$
The zeroth- and first-order shape integrals always consist of integrals over the components of $\boldsymbol{s}_{P_{0}}$ and $\Phi$ which can be precomputed for a given flexible body. We note that the precomputed shape integrals can in turn be obtained from intermediate integrals (e.g., the $S_{*}$ and
$N_{*}$ terms introduced by Wallrapp (Wallrapp, 1994), or the $\sigma, \Sigma, \Upsilon, \Psi$ terms introduced by Shabana (Shabana, 2013)). The zeroth- and first-order shape integrals are stored using a "Taylor" object-oriented class in the SI-standard input data format defined by Wallrapp. The subtlety lays in the fact that the " 1 " terms will be different if the displacement is developed using a first-order expansion or a second-order expansion. Some terms involving $\Phi_{j k}^{1}$ will be present in the latter case. The reader is referred to Wallrapp (1993) for a full deseription of the Taylor-expanded terms. Setting $\Phi_{j k}^{1}=0$ in these expressions will lead to the 1 st-order shape integral approaeh YAMS library can compute the shape integrals using a direct integration or using a finite-element formulation (see Schwertassek and Wallrapp (1999)).

The geometric stiffness introduced in Appendix C is linear in the elastic degrees of freedom $\boldsymbol{q}_{\mathrm{e}}$. Therefore, the unit geometric stiffness matrices (which are also shape integrals) can be conveniently added into the first-order terms of Shabana. Equation D2. For instance, if we write $\boldsymbol{M}_{e x}$ (given in Equation A6) using a first-order expansion, $\boldsymbol{M}_{e x}=\boldsymbol{M}_{e x}^{0}+\boldsymbol{M}_{e x}^{1} \boldsymbol{q}_{e}$, then the geometric stiffening effect can directly be inserted into the first-order term, such that $\boldsymbol{M}_{e x}^{1}$ becomes $\boldsymbol{M}_{e x}^{1}+\boldsymbol{K}_{g t}$. Similarly, the term $\boldsymbol{K}_{g r}$ can be inserted into $\boldsymbol{M}_{\theta e}^{1}, \boldsymbol{K}_{g \omega}$ into $\boldsymbol{O}_{g}^{1}, \boldsymbol{K}_{g E}$ into $\boldsymbol{\Phi}^{1}$, and $\boldsymbol{K}_{g \tau}$ into $\boldsymbol{\Psi}^{1}$ in the calculation of the generalized forces. The different contributions are summarized in Table 6.9 of the book of Schwertassek and Wallrapp (1999). A shortcoming of inserting the geometric stiffness effects into the first-order coefficient is that it could make the mass matrix symmetric (if the user code assumes $\boldsymbol{M}_{x e}=\boldsymbol{M}_{t x}^{t}$ ), instead of acting only on the third block-row of the mass matrix.

## D4 Taylor expansion of the displacement field

In the work of Wallrap (Wallrapp, 1993, 1994), the displacement field is assumed to be a function of the degrees of freedom, $\boldsymbol{u}=\boldsymbol{\Phi}_{u}\left(\boldsymbol{q}_{e}\right) \boldsymbol{q}_{e}$ where $\Phi_{u}$ consists of a Taylor series expansion of the shape functions that contain $\Phi^{0}$ and $\Phi^{1}$ terms. The resulting equations of motion are still expressed using shape integrals of the form given in Equation D2, but the 1 terms will contain some additional integrals over $\Phi^{1}$. The advantage of this method is that the $\Phi^{1}$ terms effectively account for the geometric stiffness. In practice, it is equivalent, and as convenient, to neglect the $\Phi^{1}$ terms and introduce the geometric stiffness using the method presented in Appendix $C$ (and optionally integrate them into the 1 terms as presented in Appendix D3).

D5 ElastoDyn and the partial loads approach

The ElastoDyn module of OpenFAST (Jonkman et al. 2021) uses the so-called "partial loads" approach to implement the equations of motion. The underlying theory used to derive the equations of motion is the same as Kane's formalism presented in section 2, but the partial load approach takes advantage of the fact that the calculation of reaction loads or point loads at body extremities requires similar terms to the ones needed for the equations of motion. In the discussion below, we assume that the different bodies of the structure form a tree structure with the root at the bottom and the leaves above. For a tree-like structure, there is a natural relationship between loads in the structure and the degrees of freedom. A virtual displacement of a given degree of freedom will only displace the structure above it. The equation of motion of this degree of freedom can therefore be obtained from the virtual work of the loads at a point located just above the degree of freedom, as if the entire structure above was replaced by lumped loads. The point loads contain contributions from the external loads above the point in consideration, but also inertial and gyroscopic loads associated with all the degrees of freedom of the system. If the point is at a joint, the loads corresponds to the reaction loads at this point. We write $P$ the point located after a given degree of freedom $r$. The equation of motion for this degree of freedom is obtained as if the system was isolated:
$\mathrm{f}_{r}+\mathrm{f}_{r}^{*}=0=\boldsymbol{J}_{v_{P}, r} \cdot \boldsymbol{f}_{P}+\boldsymbol{J}_{\omega_{P}, r} \cdot \boldsymbol{\tau}_{P}+h_{r}$
where: $\boldsymbol{J}_{v_{D}, x}$ and $\boldsymbol{J}_{\omega_{D}, x}$ are the partial velocities of point $P$ with respect to the degree of freedom $r ; \boldsymbol{f}_{\mathcal{D}}$ and $\boldsymbol{\tau}_{P}$ are 3-vectors containing the force and torque from the structure above the degree of freedom $r$ (including external and inertial contributions); and $h_{r}$ is the generalized load associated with the isolated degree of freedom $r$ (e.g., the elastic loads for a flexible body, or the spring and damping loads for a degree of freedom representing a joint). The point loads $f_{R}$ and $\tau_{P}$ can be decomposed into terms that are proportional to the accelerations of all the degrees of freedom (indexed with $r$ ) and additional terms (labeled " $t$ "):
$\boldsymbol{f}_{P}=\sum_{j=1}^{n_{q}} \boldsymbol{f}_{P, j} \ddot{q}_{j}+\boldsymbol{f}_{P, t}, \quad \boldsymbol{\tau}_{P}=\sum_{j=1}^{n_{q}} \boldsymbol{\tau}_{P, j} \ddot{q}_{j}+\boldsymbol{\tau}_{P, t}$
The terms $\boldsymbol{f}_{P_{x}}$ and $\boldsymbol{\tau}_{P x}$ act as generalized masses and they are referred to as "partial loads". Combining Equation D5 and Equation D6, the term $r j$ of the mass matrix and the term $r$ of the right hand side of the equation of motion (Equation 22) are obtained as:
$M^{M_{r j}=-\boldsymbol{J}_{v_{P}, r} \cdot \boldsymbol{f}_{P, j}-\boldsymbol{J}_{\omega_{P}, r} \cdot \boldsymbol{\tau}_{P, j}, \quad F_{r}=\boldsymbol{J}_{v_{P}, r} \cdot \boldsymbol{f}_{P, t}+\boldsymbol{J}_{\omega_{P}, r} \cdot \boldsymbol{\tau}_{P, t}+h_{r}}$
Therefore, the knowledge of the partial loads and the partial velocities at key points of the structure (typically, points where user outputs are desired) can be used to obtain the reaction loads (Equation D6) and the equations of motion (Equation D7). This is the approach used in ElastoDyn: the loads at key points of the structure were derived using hand calculations, and then the partial loads were used for the implementation of the outputs and the equations of motion. The reader is referred to the notes provided in the online documentation of ElastoDyn for more details (Jonkman et al., 2021). A general procedure to obtain partial loads can be devised (using kinematics to find velocities and acceleration in the structure, and computing the loads from the tree top to the root), but would be beyond the scope of this article.

## Appendix E: Equations of motion of simple wind turbine models

In this section, we present the equations of motion for the examples presented in section 4 .

## E1 Two-degrees-of-freedom model of a land-based or fixed-bottom wind turbine

In this section ${ }_{2}$ we provide some intermediate values to obtain the equations of motion given in - subsection 4.4 . We use the hat notation to indicate unit vectors of a frame, where the frame is identified as $t, n, r$ for the tower, nacelle, and rotor, respectively. For instance, $v \hat{t}_{x}$ is the unit vector in the $x$ direction of the tower frame. The degrees of freedom are $\boldsymbol{q}=(q, \psi)$. The kinematics of the tower (at its origin) are zero:
$\boldsymbol{v}_{O, T}=\mathbf{0}, \quad \boldsymbol{\omega}_{T}=\mathbf{0}, \quad \boldsymbol{a}_{O, T}=\mathbf{0}$

All Jacobians are zero except $\boldsymbol{J}_{e, 1 T}=1$ The inertial force, torque, and elastic force are:
$\boldsymbol{f}_{T}^{*}=C_{t T x} \ddot{q} \hat{\boldsymbol{t}}_{x}+M_{T} g \hat{\boldsymbol{t}}_{z}, \quad \boldsymbol{\tau}_{T}^{*}=C_{r T y} \ddot{q} \hat{\boldsymbol{t}}_{y}, \quad \boldsymbol{E}_{T}^{*}=f_{e}+D_{e} \dot{q}+\left(K_{e}+K_{q}\right) q+M_{e} \ddot{q}$
The nacelle kinematics (at its center of mass) are:
$\boldsymbol{v}_{G, N}=\dot{q} \hat{\boldsymbol{t}}_{x}+\nu_{y} z_{N G} \dot{q} \hat{\boldsymbol{n}}_{x}-\nu_{y} x_{N G} \dot{q} \hat{\boldsymbol{n}}_{z}, \quad \boldsymbol{\omega}_{N}=\nu_{y} \dot{q} \hat{\boldsymbol{t}}_{y}$
$\boldsymbol{a}_{G, N}=\ddot{q} \hat{\boldsymbol{t}}_{x}+\left(-\nu_{y}^{2} x_{N G} \dot{q}^{2}+\nu_{y} z_{N G} \ddot{q}\right) \hat{\boldsymbol{n}}_{x}+\left(-\nu_{y}^{2} z_{N G} \dot{q}^{2}-\nu_{y} x_{N G} \ddot{q}\right) \hat{\boldsymbol{n}}_{z}$

The Jacobians with respect to $q$ are:
$\boldsymbol{J}_{v, 1 N}=\hat{\boldsymbol{t}}_{x}+\nu_{y} z_{N G} \hat{\boldsymbol{n}}_{x}-\nu_{y} x_{N G} \hat{\boldsymbol{n}}_{z}, \quad \boldsymbol{J}_{\omega, 1 N}=\nu_{y} \hat{\boldsymbol{t}}_{y}$
$\boldsymbol{v}_{G, R}=\dot{q} \hat{\boldsymbol{t}}_{x}+\nu_{y} z_{N R} \dot{q} \hat{\boldsymbol{n}}_{x}-\nu_{y} x_{N R} \dot{q} \hat{\boldsymbol{n}}_{z}, \quad \boldsymbol{\omega}_{R}=\dot{\psi} \hat{\mathbf{e}}_{\mathbf{r}_{\mathbf{x}}}+\nu_{y} \dot{q} \hat{\boldsymbol{t}}_{y}$
$\boldsymbol{a}_{G, R}=\ddot{q} \hat{\boldsymbol{t}}_{x}+\left(-\nu_{y}^{2} x_{N R} \dot{q}^{2}+\nu_{y} z_{N R} \ddot{q}\right) \hat{\boldsymbol{n}}_{x}+\left(-\nu_{y}^{2} z_{N R} \dot{q}^{2}-\nu_{y} x_{N R} \ddot{q}\right) \hat{\boldsymbol{n}}_{z}$
713 The corresponding Jacobians with respect to $q$ (" 1 ") and $\psi(" 2$ ") are:
$\boldsymbol{J}_{v, 1 R}=\hat{\boldsymbol{t}}_{x}+\nu_{y} z_{N R} \hat{\boldsymbol{n}}_{x}-\nu_{y} x_{N R} \hat{\boldsymbol{n}}_{z}, \quad \boldsymbol{J}_{\omega, 1 R}=\nu_{y} \hat{\boldsymbol{t}}_{y}, \quad \boldsymbol{J}_{\omega, 2 R}=\hat{\boldsymbol{r}}_{x}$
The inertial force and torque on the nacelle are:
$\boldsymbol{f}_{N}^{*}=M_{N} \ddot{q} \hat{\boldsymbol{t}}_{x}+M_{N}\left(-\nu_{y}^{2} x_{N G} \dot{q}^{2}+\nu_{y} z_{N G} \ddot{q}\right) \hat{\boldsymbol{n}}_{x}+M_{N}\left(-\nu_{y}^{2} z_{N G} \dot{q}^{2}-\nu_{y} x_{N G} \ddot{q}\right) \hat{\boldsymbol{n}}_{z}, \quad \boldsymbol{\tau}_{N}^{*}=J_{y, N} \nu_{y} \ddot{q} \hat{\boldsymbol{n}}_{y}$
The kinematics of the rotor are:

The inertial force and torque on the rotor are:
$\boldsymbol{f}_{R}^{*}=M_{R} \ddot{q} \hat{\boldsymbol{t}}_{x}+M_{R}\left(-\nu_{y}^{2} x_{N R} \dot{q}^{2}+\nu_{y} z_{N R} \ddot{q}\right) \hat{\boldsymbol{n}}_{x}+M_{R}\left(-\nu_{y}^{2} z_{N R} \dot{q}^{2}-\nu_{y} x_{N R} \ddot{q}\right) \hat{\boldsymbol{n}}_{z}$
$\boldsymbol{\tau}_{R}^{*}=J_{x, R} \ddot{\psi} \hat{\boldsymbol{r}}_{x}$
$+\left(J_{\oplus, R} \nu_{y} \sin (\psi) \dot{\psi} \dot{q}+J_{\oplus, R}\left(-\nu_{y} \sin (\psi) \dot{\psi} \dot{q}+\nu_{y} \cos (\psi) \ddot{q}\right)-J_{x, R} \nu_{y} \sin (\psi) \dot{\psi} \dot{q}\right) \hat{\boldsymbol{r}}_{y}$
$+\left(J_{\oplus, R} \nu_{y} \cos (\psi) \dot{\psi} \dot{q}+J_{\oplus, R}\left(-\nu_{y} \sin (\psi) \ddot{q}-\nu_{y} \cos (\psi) \dot{\psi} \dot{q}\right)-J_{x, R} \nu_{y} \cos (\psi) \dot{\psi} \dot{q}\right) \hat{\boldsymbol{r}}_{z}$

## E2 Three-degrees-of-freedom model of a land-based or fixed-bottom wind turbine

The equations of motion for the model presented in subsection 4.5 , with $\boldsymbol{q}=\left(q_{1}, q_{2}, \psi\right)$, are given in this section. The elements of the mass matrix are:
$M_{11}=\left[M_{e 11}+M_{N}+M_{R}\right]$
$+\left[J_{y, N}+J_{\oplus, R}+M_{N}\left(x_{N G}^{2}-2 x_{N G} q_{1}+z_{N G}^{2}\right)+M_{R}\left(x_{N R}^{2}-2 x_{N R} q_{1}+z_{N R}^{2}\right)\right] \nu_{y}^{2}$
$+2\left[M_{N} z_{N G}+M_{R} z_{N R}\right] \nu_{y}$
$M_{13}=J_{x, R} \theta_{t} \nu_{x} \nu_{y} q_{2}$
$M_{22}=\left[M_{e 22}+M_{N}+M_{R}\right]$
$+\left[J_{x, N}+J_{x, R}+M_{N} z_{N G}^{2}+M_{R} z_{N R}^{2}\right] \nu_{x}^{2}$
$-2\left[M_{N} z_{N G}+M_{R} z_{N R}\right] \nu_{x}$
$M_{23}=J_{x, R} \nu_{x}$
$M_{33}=J_{x, R}$

The elements of the forcing vector are:

```
\(f_{1}=f_{e 1}-K_{e 11} q_{1}-D_{e 11} \dot{q}_{1}-J_{x, R} \theta_{t} \nu_{x} \nu_{y} \dot{\psi} \dot{q}_{2}+\left[M_{N} x_{N G}+M_{R} x_{N R}\right] \nu_{y}^{2} \dot{q}_{1}^{2}\)
    \(+g\left[M_{N}\left(\nu_{y}^{2} z_{N G} q_{1}+\nu_{y} x_{N G}\right)+M_{R}\left(\nu_{y}^{2} z_{N R} q_{1}+\nu_{y} x_{N R}\right)\right]+f_{a}\left[\theta_{t} \nu_{y} x_{N R}-\theta_{t} \nu_{y} q_{1}+\nu_{y} z_{N R}+1\right]\)
\(f_{2}=f_{e 2}-K_{e 22} q_{2}-D_{e 22} \dot{q}_{2}+J_{x, R} \theta_{t} \nu_{x} \nu_{y} \dot{\psi} \dot{q}_{1}\)
    \(+g\left[M_{N} z_{N G}+M_{R} z_{N R}\right] \nu_{x}^{2} q_{2}+f_{a} \theta_{t} \nu_{x} q_{2}\)
\(f_{3}=-J_{x, R} \theta_{t} \nu_{x} \nu_{y} \dot{q}_{1} \dot{q}_{2}+\tau_{a}\)
\(f_{2}=f_{e 2}-K_{e 22} q_{2}-D_{e 22} \dot{q}_{2}+J_{x, R} \theta_{t} \nu_{x} \nu_{y} \dot{\psi} \dot{q}_{1}\)
\(+g\left[M_{N} z_{N G}+M_{R} z_{N R}\right] \nu_{x}^{2} q_{2}+f_{a} \theta_{t} \nu_{x} q_{2}\)
\(f_{3}=-J_{x, R} \theta_{t} \nu_{x} \nu_{y} \dot{q}_{1} \dot{q}_{2}+\tau_{a}\)
```


## E3 Three-degrees-of-freedom model of a floating wind turbine

The equations of motion for the model presented in subsection 4.6 , with $\boldsymbol{q}=\left(x, \phi, q_{T}\right)$, are given in this section. The elements of the mass matrix are:
$M_{11}=M_{F}+M_{T}+M_{N}$
$M_{12}=M_{F} z_{F G}-M_{d T z}+M_{N}\left[L_{T}+z_{N G}-\nu_{y} x_{N G} q_{T}-\phi_{y}\left(x_{N G}+q_{T}+\nu_{y} z_{N G} q_{T}\right)\right]$
$M_{13}=C_{t T 1 x}+M_{N}\left[1+\nu_{y} z_{N G}-\nu_{y}^{2} x_{N G} q_{T}-\phi_{y}\left(\nu_{y}^{2} z_{N G} q_{T}+\nu_{y} x_{N G}\right)\right]$
$M_{22}=J_{y, F}+M_{F} z_{F G}^{2}+J_{T, y}+J_{y, N}+M_{N}\left[\left(L_{T}^{2}+z_{N G}\right)^{2}+\left(q_{T}+x_{N G}\right)^{2}+2 \nu_{y} q_{T}\left(z_{N G} q_{T}-L_{T} x_{N G}\right)\right]$
$M_{23}=C_{r T 1 y}+\left[J_{y, N}+M_{N}\left(x_{N G}^{2}+z_{N G}^{2}+L_{T} z_{N G}+\nu_{y} q_{T}\left(z_{N G} q_{T}-L_{T} x_{N G}\right)\right] \nu_{y}+M_{N}\left[L_{T}+z_{N G}\right]\right.$
$M_{33}=M_{e}+M_{N}+\left[J_{y, N}+M_{N}\left(x_{N G}^{2}-2 x_{N G} q_{T}+z_{N G}^{2}\right)\right] \nu_{y}^{2}+2 M_{N} \nu_{y} z_{N G}$
The elements of the forcing vector are:

$$
\begin{align*}
f_{1} & =f_{H}+\left[M_{F} z_{F G}-M_{d z}+M_{N}\left(L_{T}+z_{N G}-\nu_{y} x_{N G} q_{T}\right)\right] \phi_{y} \dot{\phi}_{y}^{2}+M_{N}\left[q_{T}+x_{N G}+\nu_{y} z_{N G} q_{T}\right] \dot{\phi}_{y}^{2}  \tag{E33}\\
& +\left[2 C_{t x}+M_{N}\left(1+\nu_{y} z_{N G}-\nu_{y}^{2} x_{N G} q_{T}\right)\right] \phi_{y} \dot{\phi}_{y} \dot{q}_{T}+M_{N} \nu_{y}\left[x_{N G}+\nu_{y} z_{N G} q_{T}\right] \dot{\phi}_{y} \dot{q}_{T}  \tag{E34}\\
& +M_{N} \nu_{y}^{2}\left[x_{N G}+z_{N G} \phi_{y}\right] \dot{q}_{T}^{2}  \tag{E35}\\
& +f_{a}\left[1-\theta_{t} \nu_{y} q_{T}-\nu_{y} \phi_{y} q_{T}\right]  \tag{E36}\\
f_{2} & =\tau_{H}+M_{N}\left[\nu_{y}^{2}\left(L_{T} x_{N G}-z_{N G} q_{T}\right)\right] \dot{q}_{T}^{2}  \tag{E37}\\
& -2 M_{N}\left[q_{T}+x_{N G}+\nu_{y}\left(2 z_{N G} q_{T}-L_{T} x_{N G}\right)-\nu_{y}^{2} q_{T}\left(L_{T} z_{N G}+x_{N G} q_{T}\right)\right] \dot{\phi}_{y} \dot{q}_{T}  \tag{E38}\\
& +g\left[M_{F} z_{F G} \phi_{y}-M_{d z} \phi_{y}+M_{N}\left\{\left(L_{T}+z_{N G}-\nu_{y} x_{N G} q_{T}\right) \phi_{y}+q_{T}+x_{N G}+\nu_{y} z_{N G} q_{T}\right\}\right]  \tag{E39}\\
& +f_{a}\left[L_{T}+z_{N R}+\theta_{t} x_{N R}+\theta_{t} q_{T}+\nu_{y} q_{T}^{2}-L_{T} \theta_{t} \nu_{y} q_{T}\right]  \tag{E40}\\
f_{3} & =f_{e}-D_{e} \dot{q}_{T}-K_{e} q_{T}  \tag{E41}\\
& +M_{N}\left[q_{T}+x_{N G}+\nu_{y}\left(2 z_{N G} q_{T}-L_{T} x_{N G}\right)-\nu_{y}^{2} q_{T}\left(L_{T} z_{N G}+x_{N G} q_{T}\right)\right] \dot{\phi}_{y}^{2}  \tag{E42}\\
& +M_{N} \nu_{y}^{2} x_{N G} \dot{q}_{T}^{2}  \tag{E43}\\
& +g\left[C_{t T 1 x} \phi_{y}+M_{N}\left(\nu_{y} x_{N G}+\nu_{y}^{2} z_{N G} q_{T}-\nu_{y}^{2} x_{N G} \phi_{y} q_{T}+\nu_{y} z_{N G} \phi_{y}+\phi_{y}\right)\right]  \tag{E44}\\
& +f_{a}\left[1+\theta_{t} \nu_{y} x_{N R}-\theta_{t} \nu_{y} q_{T}+\nu_{y} z_{N R}\right] \tag{E45}
\end{align*}
$$

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[^0]:    ${ }^{1}$ We address the second-order first-order approximation in and-Appendix D4.

[^1]:    ${ }^{2}$ The relevant equations of the shape function approach for a beam are given in Appendix B.

